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Quantifying The Effectiveness of a Grout Curtain Using a Laboratory-Scale Physical Model

Elliot N. Magoto

University of Kentucky, elliott.magoto@gmail.com

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Elliot N. Magoto, Student

Dr. L. Sebastian Bryson, Major Professor

Dr. Ed Wang, Director of Graduate Studies

QUANTIFYING THE EFFECTIVENESS OF A GROUT CURTAIN USING A
LABORATORY-SCALE PHYSICAL MODEL

THESIS

A thesis submitted in partial fulfillment of the requirements for the degree of Master of
Science in Civil Engineering in the College of Engineering at the University of Kentucky

By

Elliot Nicholas Magoto

Lexington, Kentucky

Director: Dr. L. Sebastian Bryson, Associate Professor of Civil Engineering

Lexington, Kentucky

2014

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ABSTRACT

QUANTIFYING THE EFFECTIVENESS OF A GROUT CURTAIN USING A LABORATORY-SCALE PHYSICAL MODEL

In the past decade, the grouting industry has made significant technological advancements in real-time monitoring of flow rate and pressure of pumped grout, stable grout mix design, and with grout curtain concepts dealing with placement and orientation. While these practices have resulted in improved construction practices in the grouting industry, current design guidelines for grout curtains are still predominately based on qualitative measures such as engineering judgment and experience or are based on proprietary methods. This research focused on the development of quantitative guidelines to evaluate the effectiveness of a grout curtain in porous media using piezometric and hydraulic flow data. In this study, a laboratory-scale physical seepage model was developed to aid in the understanding and development methodology to evaluate the effectiveness of a grout curtain. A new performance parameter was developed based on a normalization scheme that utilized the area of the grout curtain and the area of the improved media. The normalization scheme combined with model-based Lugeon values that correspond to pore pressure and flow rate measurements at different soil unit weights and grout curtain spacings, produced a mathematical equation that can be used to quantify the effectiveness of a grout curtain. This study found a relationship that takes into account soil unit weight, grout curtain spacing and a new performance parameter that can be used to help predict the effectiveness of a grout curtain.

KEYWORDS: Grout Curtain; Seepage Cutoff; Lugeon Value; Hydraulic Conductivity; Pore Pressure

Elliot Nicholas Magoto

3/10/2014

QUANTIFYING THE EFFECTIVENESS OF A GROUT CURTAIN USING A
LABORATORY-SCALE PHYSICAL MODEL

By

Elliot Nicholas Magoto

L. Sebastian Bryson, Ph.D., P.E.

Director of Thesis

Ed Wang, Ph.D., P.E.

Director of Graduate Studies

3/10/2014

To

My Parents: Nicholas and Jill Magoto

My Brother: Zach Magoto

My Grandparents: Pete and Helen Magoto, Purcell and Louise Grilliot

Girlfriend: Kelsey Peters

For all their love, support and guidance.

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Chapter 1: Introduction

Grout curtains, also known as cut-off walls are vertically drilled tangent shafts that are filled with cementitious material that create a barrier to help prevent excessive seepage under a dam. Grout curtains are intended to be impervious walls that typically exist below infrastructure to minimize water seepage. Along with seepage minimization under dams, grout curtains are also found in karst terrain to minimize groundwater infiltration and subsequent erosion of the geologic formation. A generalized depiction of a grout curtain wall can be seen in Figure 1.1.

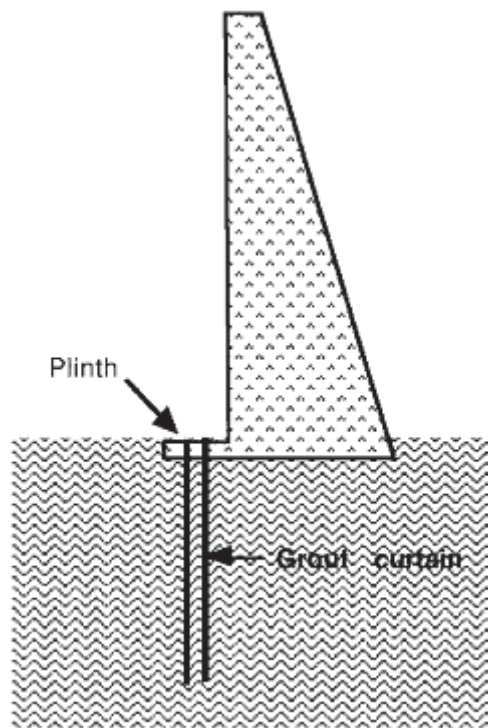


Figure 1.1: Schematic of general grout curtain wall (Weaver, K. and Bruce, D., 2007).

Dams dating back to the 1900's were predominately constructed in geologic areas consisting of karst terrain. Karst terrain is the resulting landform that is produced when

groundwater dissolves and washes away existing rock material within the underlying soil structure. Cracks, voids, sinkholes and caves are a result of this dissolving behavior. The materials susceptible to dissolving include rock such as limestone, dolomite and gypsum according to research done by Veni et al. (2001). This dissolving behavior can cause significant problems with the structural integrity of various impoundments, specifically dams. Dams located in karst terrain often times deal with significant groundwater seepage issues. Given sufficient time, groundwater will eventually erode enough of the underlying soil structure to create conditions where dam failures can occur. To prevent dam failures from occurring in karst terrain, grout curtains are typically installed. As previously mentioned, grout curtains are installed to prevent excessive seepage under dams and to minimize groundwater infiltration.

In the past decade, the grouting industry has made significant technological advancements in real-time monitoring of flow rate and pressure of pumped grout, stable grout mix design, and with grout curtain concepts dealing with placement and orientation. While these practices have resulted in a renewed appreciation of the grouting industry, current design guidelines are still predominately based on qualitative measures such as engineering judgment, experience and rules-of-thumb. Not being able to use quantifiable means to assess the effectiveness of a grout curtain, has left many engineers spending lots of valuable time and money. Even though previous studies in the past few decades such as the studies conducted by Uromeihy and Farrokhi (2012) and Ajalloeian et al. (2012) both of which evaluate the groutability of Iranian dams have shown that qualitative based approaches have worked; being able to quantify the effectiveness of a grout curtain would seem to be more beneficial.

Today, most grout curtain designs and installations are performed based on local rules-of-thumb and experience combined with adequate engineering judgment. Even though using these qualitative approaches have shown to work in the past, many engineers and designers have questioned whether or not qualitative approaches are the most efficient way in determining the effectiveness of a grout curtain. According to Bruce (1992), the utilization of qualitative approaches previously discussed is not the most efficient approach when considering the effectiveness of a grout curtain. It is believed that lots of money, time and effort are lost when performing an extensive grouting program using qualitative approaches as the predominate basis. Qualitative based approaches have shown to work with the utilization of excessive materials and time. However, with today's society combined with the economical issues encountered, clients today do not have the unlimited resources at their disposal as they once did. Thus, it is important to develop a quantitative approach to predict the effectiveness of a grout curtain prior to installation. A quantitative approach would help engineers and designers optimize grout curtain performance while minimizing factors such as time, labor and cost.

Therefore, this research focused on the development of quantitative guidelines to evaluate the effectiveness of a grout curtain design in porous media using piezometric and hydraulic flow data. In this study, a laboratory-scale physical seepage model was developed to aid in the understanding and development methodology to evaluate the effectiveness of a grout curtain. A new performance parameter was developed based on a normalization scheme that utilized the area of the grout curtain and the area of the improved media. The normalization scheme combined with model-based Lugeon values

that correspond to pore pressure and flow rate measurements at different soil unit weights and grout curtain spacings, produced a mathematical equation that can be used to quantify the effectiveness of a grout curtain. This study found a relationship that takes into account soil unit weight, grout curtain spacing and a new performance parameter that can be used to help predict the effectiveness of a grout curtain.

1.1 Proposed Concept

In order to develop a quantifiable relationship that assesses the effectiveness of a grout curtain, a new performance parameter had to be established as previously mentioned. Through much deliberation it was believed that a performance parameter similar to Priebe (1991) and Priebe (1995) area replacement ratio on vibro replacement columns for ground improvement could be utilized to help develop a new performance parameter that would aid in the quantification of a grout curtain. The area replacement ratio parameter presented in Priebe (1991) and Priebe (1995) research utilized various areas with respect to the improved and unimproved ground. This research uses an area replacement ratio combined with other parameters such as Poisson's ratio and friction angle to help determine ground improvement effects on non-compaction cohesive soils utilizing load bearing columns of well compacted coarse grained backfill. Unfortunately, no data has been presented using the area replacement ratio as a parameter to help quantify the effectiveness of a grout curtain. However, since the installation of vibro replacement columns for ground improvement is similar to the installation of grout columns for a grout curtain, a new performance parameter based on the area replacement ratio seemed applicable. Therefore, this research utilized a new performance parameter

similar to the area replacement ratio presented in Priebe (1991) and Priebe (1995) as a key component to quantifying the effectiveness of a grout curtain.

Along with the development of a new performance parameter, a laboratory-scale seepage model was an essential component to this research. The laboratory-scale seepage model will be used to investigate various piezometric and hydraulic flow data corresponding to various simulated grout curtain concepts. Previous research has been conducted using laboratory-scale models (Luofenga et al., 2012) to explore various seepage behaviors that develop with the presence of a highly permeable sand foundation. Similar to Luofenga et al., 2012 study, a research team explored seepage behavior with respect to slope stability upon failure of a dam (Awal et al., 2009). Additionally, another study (Liua et al., 2003) successfully utilized a laboratory-scale model to determine water flow patterns through foundations similar to seepage through a concrete dam. All three previous studies mentioned the difficulty in modeling highly complex geological insitu field conditions based on numerical modeling. Thus, it was concluded that laboratory-scale models were a necessity in the understanding and development of seepage behaviors with their corresponding situations. Since, it has been proven that laboratory-scale models can successfully aid in the understanding of seepage behavior, the use of a laboratory-scale seepage model seems applicable for quantifying the effectiveness of a grout curtain.

1.2 Objectives of Research

The goal of this research was to develop a laboratory-scale physical seepage model that would help elucidate general seepage behavior that could be used to establish

a means of evaluating the effectiveness of grout curtain quantitatively rather than qualitatively. Therefore, the objectives of this research were as follows:

- Analyze current methods used to assess the effectiveness of a grout curtain
- Develop a laboratory-scale physical seepage model that would help investigate specific behavior related to seepage in porous media
- Develop a new performance parameter that would aid in the quantification of effectiveness of a grout curtain
- Develop a quantifiable relationship between the performance parameter and fundamental design parameters associated with the grout curtain
- Assess the general applicability of the new quantifiable relationship using case history studies to show that the quantifiable relationship can be scaled

1.3 Relevance of Research

As mentioned previously, lots of money and time are spent on developing and performing extensive grouting programs to improve the integrity of dams and impoundments based on qualitative measures. The current methods used to assess the effectiveness of a grout curtain, combined with today's current economical issues have created a need for a quantifiable relationship to assess the effectiveness of a grout curtain. Therefore, this research was established to develop a quantifiable relationship to assess the effectiveness of a grout curtain that could be used by engineers to help minimize factors such as time, cost and labor while providing an adequate solution. The quantifiable relationship that assesses the effectiveness of a grout curtain can be used to predict the behavior of simulated grout curtains prior to installation. This relationship

allows engineers to have more control while optimizing the design of a grout curtain. This relationship avoids the use of rules-of-thumb and qualitative approaches currently used by industry to assess the effectiveness of a grout curtain. It is believed that this quantifiable relationship can also be used in ground improvement.

1.4 Contents of Thesis

Chapter 2 introduces the idea of grout curtains and current methodologies used to assess the effectiveness of grout curtains which entail Lugeon, hydraulic conductivity and pore pressure values. Accompanying this information, case histories are presented that are the primary bases for which the quantifiable relationship presented in this research is verified.

Chapter 3 describes the test materials used in this research. Along with test material descriptions, the development and description of the laboratory-scale physical seepage model is discussed herein.

Chapter 4 describes the associated test procedures that aided in the development of a quantifiable relationship. Test data and results associated with the testing procedures are presented as well. The development of a new performance parameter is described and addressed. Chapter 4 also presents the analysis that was performed during this research. This chapter describes and evaluates the trends observed during the experimentation portion of this research. These generalized trends were used in the development of a quantifiable relationship that can be used in assessing the effectiveness of a grout curtain.

Chapter 5 presents case history data that was used to verify the new quantifiable relationship. Case history studies will help assist in determining the accuracy of this relationship.

Chapter 6 recaps the discussion presented in this research in its entirety. Final conclusions brought forth by this research are summarized and discussed. A brief discussion of future research recommendations is presented.

Chapter 2: Literature Review

2.1 Introduction to Grout Curtains

Groundwater seepage under dams and other impoundments is a significant problem that geotechnical engineers are encountering more and more every day. As dams continue to age, groundwater seepage issues are becoming more prevalent. If groundwater seepage issues are left unaccounted for, significant problems can cultivate. Past history has shown that groundwater seepage under dams and other impoundments can cause significant problems not just for the owner, but to the surrounding communities. Problems associated with prolonged groundwater seepage under dams and impoundments have led to breaches. For example, the Teton Earthen Dam located in the eastern part of Idaho failed due to excessive seepage through the earth fill dam. The permeable loess core material combined with rock fissures along the abutments of the dam, allowed for significant seepage through and around the dam (Arthur, 1977). Figure 2.1, shows a picture of the Teton Earthen Dam failure.



Figure 2.1: Teton Earthen Dam Failure in 1976 (Arthur, 1977).

The seepage through the earthen dam caused structural degradation ultimately leading to the breach of the dam. Tremendous damage resulted in the breach of the Teton Earthen Dam. Significant flooding occurred in the communities just downstream of the dam. Damages were estimated to be nearly one billion dollars. Unfortunately, the flooding due to the breach of the dam claimed 14 lives. As can be seen, groundwater seepage under dams can introduce significant issues to the structural integrity of dams and other impoundments.

To avoid such horrific circumstances like the Teton Earthen Dam failure, the use of grout curtains has been proven to successfully mitigate groundwater seepage issues that are typically encountered at dams and other impoundments. Grout curtain walls are being incorporated in more dam construction designs since it is currently the most effective approach to mitigating seepage problems. Grout curtains are a cost effective way to diffuse seepage issues due to the low cost of the grout material. It is important to note that prior to the 1950's steel sheet piles were utilized to create impermeable walls similar to grout curtains as pointed out by Powers et al. (2007). However, literature (Powers et al., 2007) has shown that grout curtain walls outperform impermeable walls constructed of steel sheet piles with respect to factors such as cost and seepage mitigation. The mere cost of steel has precluded the use of steel sheet piles as a viable solution for seepage mitigation.

2.2 Grout Installation Techniques

There are three primary grout installation techniques that are widely used today in the civil engineering industry. The three grout curtain techniques that are commonly used

today which are highlighted in the research done by Yong-Jiang and Xing-Wang (2012) include jet-grouting, high-mobility grouting, and compaction grouting. The goal of all three installation techniques is to simply prevent seepage from occurring under water-retaining structures, specifically dams.

Jet-grouting is a technique that uses high velocity and high pressure jets to hydraulically replace poor rock or in-situ soil material with a cementitious material known as grout. Specialized machinery connected to a grout monitoring system allow for the placement of grout. The process of jet-grouting is fairly simplistic. High velocity grout jets connected to a drill-stem allow for insitu soil to be eroded then mixed. The composition of insitu soil and grout is commonly referred as soilcrete (Hayward Baker, 2014). Figure 2.2 illustrates the installation of a soilcrete column using jet-grouting.

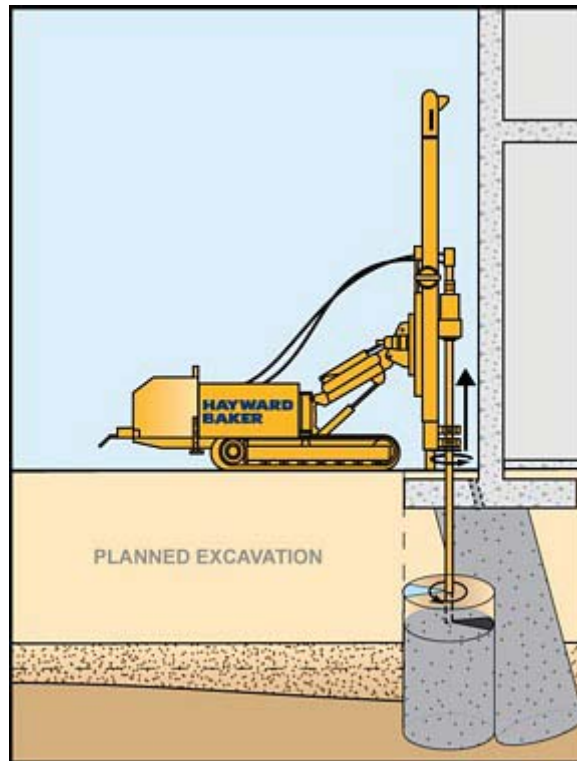


Figure 2.2: Installation of a soilcrete column using jet-grouting (Hayward Baker, 2014).

Given adequate time for set-up, the soilcrete columns cure and become high strength, low permeability material. Soilcrete columns are installed at predetermined locations where seepage issues are expected. Soilcrete columns are installed to help prevent groundwater seepage from occurring under dams and other impoundments. This technique is very versatile since grouting can take place above or below the ground water table and can be used in a wide range of soils from high plasticity clays to cohesionless sands.

High-mobility grouting uses the flow of a pressurized cementitious grout material. Over time, the grouting material enters into the crevices of the underlying soil causing the grout and soil to bind together. Figure 2.3 presents the high-mobility grouting technique.

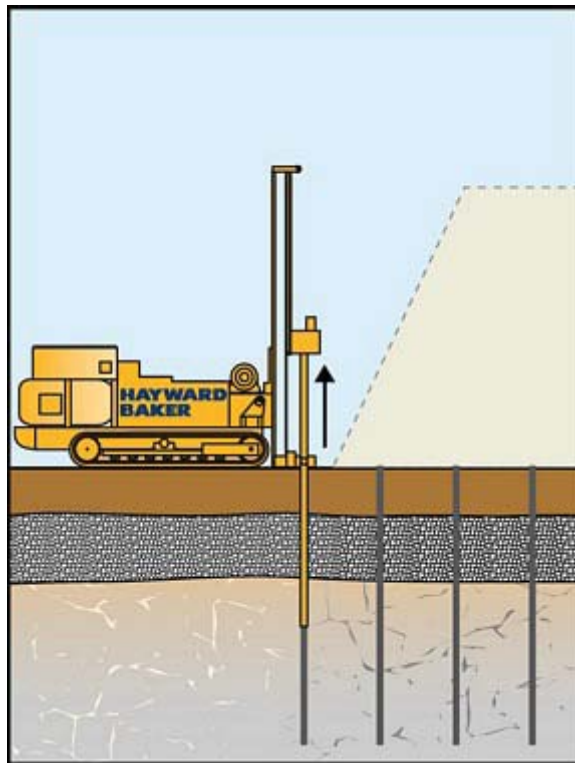


Figure 2.3: Illustration of the high-mobility grouting technique (Hayward Baker, 2014).

When performing high-mobility grouting for dams or other impoundments, it is imperative that the size of the pores or void spaces of the underlying soil material are matched to the particle size of the grout being applied. Having the appropriate grout with respect to particle size will allow for the grout material to enter the pore and void spaces of the underlying soil material. If the particle size of the grout is larger than the pore and void spaces of the underlying soil, grout will be unable to enter the pore and void spaces. This type of grouting allows for increased strength properties such as cohesion, as well as decreased permeability.

Compaction grouting is another common technique used by industry today. Compaction grouting utilizes low viscosity grout to displace and densify loose soils. Also, compaction grouting is performed to stabilize large void spaces known as sinkholes by using a low-mobility grout mixture (Hayward Baker, 2014). The pressurized grout is injected into the ground by a pipe. As the grout is continuously injected, the pipe is slowly raised, forming a bulb like structure. For further clarity refer to Figure 2.4, which illustrates the compaction grouting technique.

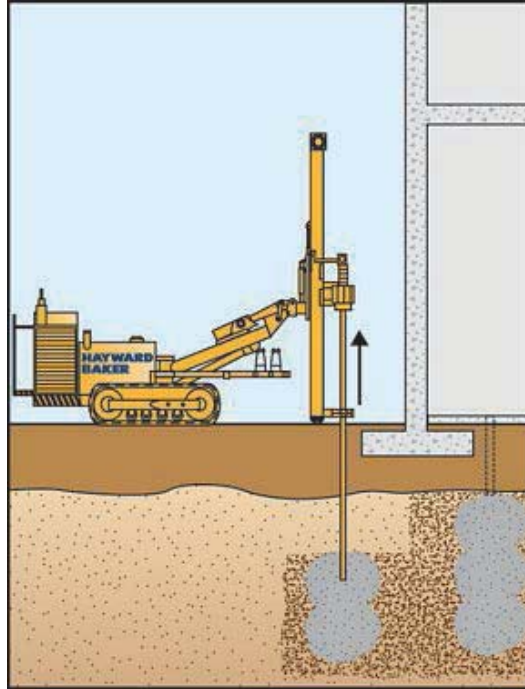


Figure 2.4: Compaction grouting technique (Hayward Baker, 2014).

The injected grout displaces the loose surrounding material. During the displacement and expansion of the grout material, geotechnical properties such as density, friction angle, and stiffness are increased. This technique reduces permeability while providing additional strength to existing underlying stratigraphy.

2.3 Current Methodologies to Assess Grout Curtain Effectiveness

2.3.1 Monitoring Lugeon Values

Monitoring Lugeon values during the installation of a grout curtain is the current state-of-the-practice for assessing grout curtain effectiveness. Lugeon values are defined as the injected volume of water in a length of time per length of rock beneath the reference elevation (Lugeon, 1933). The most common units attributed to the Lugeon value is 1 liter/minute per meter at a reference pressure of 1 MPa. Although the Lugeon

value is similar to hydraulic conductivity, the Lugeon value is typically used in rock masses in which water travels through cracks in rocks, whereas hydraulic conductivity is used in conjunction with water traveling through soil pore space. Monitoring Lugeon values has many benefits. Benefits of ascertaining Lugeon values include the determination of flow characteristics, provide a sound basis for the selection of an appropriate grout mix, and most importantly for quality control purposes (Weaver, K. and Bruce, D., 2007). Lugeon values are used combined with qualitative measures such as engineering judgment and rules-of-thumb (Bruce, 1982; Quinn et al., 2011; Berhane and Walraevens, 2013) to address the effectiveness and structural integrity of grout curtain walls. Lugeon values have been increasingly used as a means to assess quality control of grouting operations due to technological advancements in data acquisition systems and real time monitoring equipment. According to Houlsby (1976), quality assurance of a grout curtain with the use of Lugeon values is a key component to assessing the effectiveness of a grout curtain. A typical real time monitoring equipment and data acquisition system currently used in industry can be seen in Figure 2.5. The combination of real time monitoring equipment and data acquisition systems have allowed researchers (Sasaki and Tosaka, 2012; Sadeghiyeh et al., 2013) to monitor Lugeon values during grout curtain installation procedures. As the costs for real time monitoring systems and data acquisition systems start to decline due to competitive markets, Lugeon values are going to become even more widely used.



Figure 2.5: Typical real time monitoring and equipment and data acquisition system (Quinones-Rozo, 2010).

2.3.1.1 Lugeon Test Procedures

The most common insitu testing procedure used to assess the need for foundation grouting at dams and other impoundments is the Lugeon test, also known as the packer test. The original Lugeon test was developed by Maurice Lugeon in 1933. With technological advancements in real time monitoring equipment, much research has been conducted on the applicability of the original Lugeon test over the years. Research done by Hously (1976) resulted in an updated Lugeon test that allows for tests to be conducted over a wider range of pressures, while using the same principles used in the development of the original Lugeon test. The updated Lugeon test, commonly referred to as the modified Lugeon test, is the current industry standard for ascertaining Lugeon values. Unlike the original Lugeon test developed by Maurice Lugeon, the modified Lugeon test consists of 5 consecutive stages. The first three stages are completed at increasing pressures while the last two stages are completed at decreasing pressures.

During each stage, water pressure is held constant, while pumping as much water through the test interval as possible. A schematic figure illustrating a Lugeon test configuration is presented in Figure 2.6.

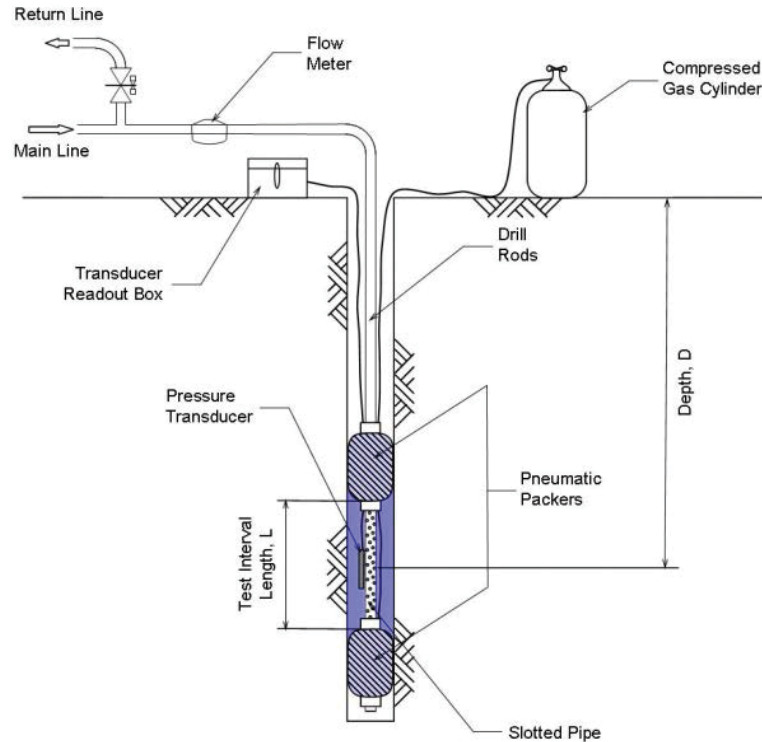


Figure 2.6: Schematic Lugeon test set-up (Quinones- Rozo, 2010).

During each stage, flow rate and pore pressure measurements are taken. These measurements are subsequently used to calculate a Lugeon value based on an equation presented in Hously (1976) and Quinones-Rozo (2010). The equation presented in Hously (1976) and Quinones-Rozo (2010) is the standard equation that is currently being used by industry to determine Lugeon test values, which can be seen in Equation 1.

$$LV = \alpha \left(\frac{q}{L} \right) \left(\frac{P_0}{P} \right) \quad (1)$$

where LV = Lugeon value, α = unit system coefficient, q = flow rate = Q/t where Q = total volume of water discharged and t = total time of test, L = test interval length of the representative test sample, P_0 = reference pressure = 1 MPa and P = water injection pressure.

The calculated Lugeon values are then assessed using Houlsby (1976) flow chart which can be seen in Figure 2.7 to determine whether or not grouting is warranted.

**WHEN IS GROUTING WARRANTED TO CONTROL LEAKAGE UNDER A DAM?
WHEN HAS ENOUGH GROUTING BEEN DONE ?**

WHEN PERMEABILITIES ARE THOSE SHOWN BELOW, OR TIGHTER.

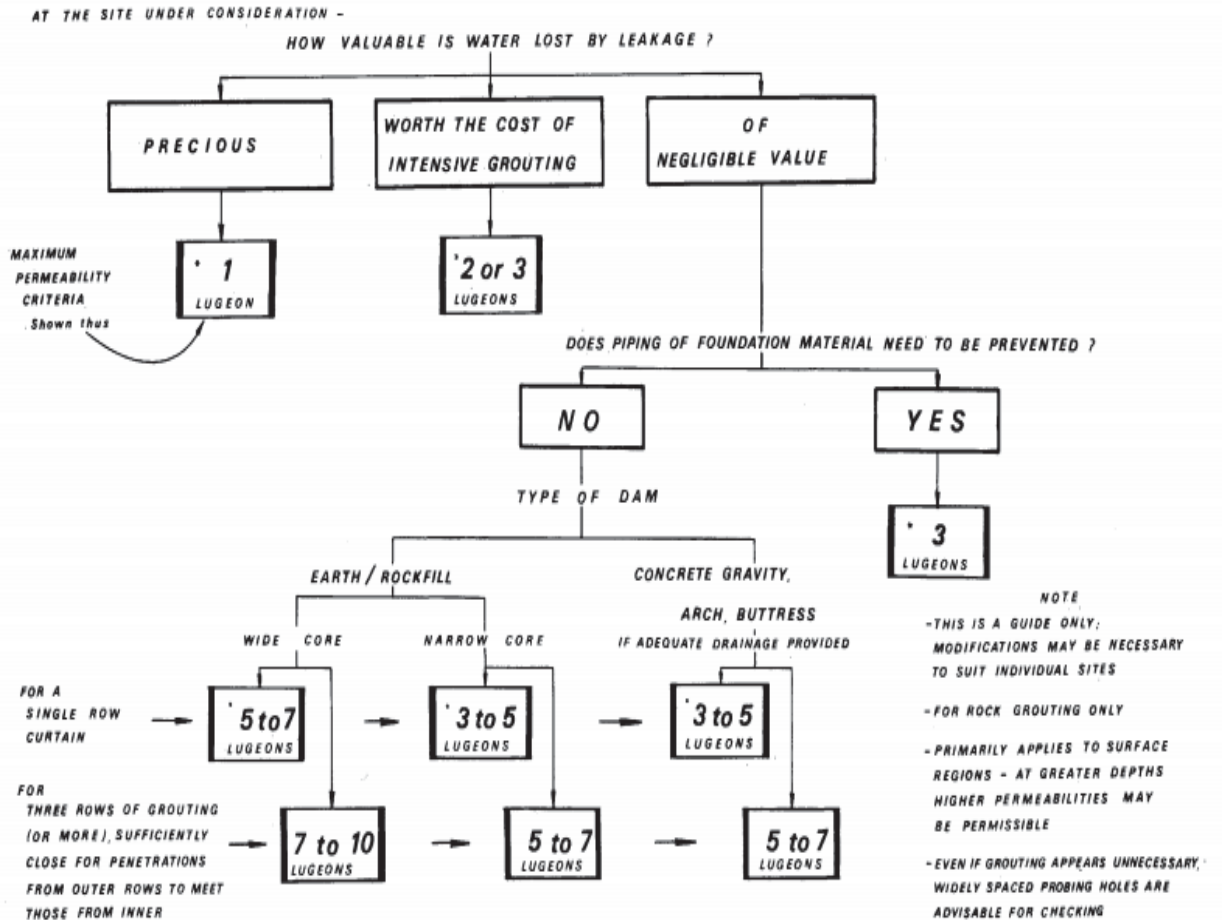


Figure 2.7: Housby's (1976) grouting applicability flow chart.

However, the use of Houlby (1976) chart is more of a qualitative based approach or rule-of-thumb rather than a quantitative based approach. Lugeon values are not the only factor when determining if grouting is appropriate. Houlby's (1976) research pointed out that other qualitative factors exist that should be taken in consideration, such as geological and other local factors. The grouting industry finds itself in an unfortunate situation where grout curtain effectiveness is determined by a qualitative based approach rather than a quantitative based approach.

2.3.1.2 Shortcoming of the Lugeon Test

Selecting an appropriate representative sample at a test site is a major drawback to the Lugeon test. The range of a single Lugeon test with a length interval of 10 feet is said to only encompass a 30 foot radius around the bore hole of interest (Bliss and Rushton, 1984). Since a Lugeon test only accurately depicts a limited area surrounding a bore hole, it is imperative to have a proper representative sample that takes into account the underlying soil material. However, if proper representative samples of the underlying soil material are obtained, Lugeon values are an appropriate measure to help aid in the quantification of the effectiveness of a grout curtain.

2.3.2 Measuring Hydraulic Conductivity

Grout curtain effectiveness is sometimes evaluated based on the degree of which in-situ hydraulic conductivity is reduced. In a previous study (Cotton and Matheson, 1990) conducted to evaluate the effectiveness of a grout curtain, hydraulic conductivity measurements were used to quantify effectiveness. In this study, effectiveness of a grout

curtain was based on hydraulic conductivity measurements with varying grout curtain depths. This study found that an effective grout curtain is not achieved until the grout curtain hydraulic conductivity values is three to four orders of magnitude less than that of the surrounding material. However, the Cotton and Matheson (1990) study used hydraulic conductivity values through a homogenous soil mass as opposed to a fractured mass often times seen while grouting.

2.3.3 Monitoring Pore Pressures

Determining the effectiveness of a grout curtain using solely pore pressure measurements is uncommon. However, one researcher compared predicted and observed behavior of pore water pressure inside an Alavian earthfill dam in Iran (Aminfar et al., 2009). This study also investigated the effects pore water pressures had on the foundation of the dam by looking at the distribution of pore water pressures. Beyond this study, there is little data that has been presented using solely pore pressures to evaluate the effectiveness of a grout curtain.

2.3.4 Grout Mix Design

Early studies evaluating the effectiveness of a grout curtain were primarily based on the performance of the grout mix design. There were several researchers (Bodocsi and Bowers, 1991; Anagnostopoulos and Kadjispyrou, 2004; Ozgurel and Vipulanandam, 2005; Lirer et al., 2006) who investigated various grout mix design parameters, such as workability, strength, and durability of the grout material under different conditions, to help evaluate the performance of grout curtains. However, these researchers did not evaluate the influence of the installation sequence of grout curtains on the overall effectiveness.

2.3.5 Willowstick

Willowstick is a recently developed technology that attempts to define and model complex subsurface water systems using electromagnetic fields (White Paper, 2012). Willowstick technology utilizes the placement of strategically placed electrodes in conjunction with a power supply to help enhance magnetic fields that can assist in modeling preferential groundwater flow paths (White Paper, 2012). Figure 2.8 illustrates modeling done by Willowstick technology. Unlike typical electromagnetic and resistivity methods, the Willowstick technology understands that flow paths can be effectively modeled using electrode probes due to their thorough understanding of water content and subsurface electrical conductivity (White Paper, 2012). This technology was developed to be a cost effective method to modeling complex subsurface water systems. The traditional method of direct observation through the drilling of wells is too time consuming, labor intensive and expensive. The Willowstick technology capitalizes on low cost and increased safety factors. However, this may be considered state-of-the-art, it is not currently widely used by consultants. The research presented herein focused on readily available technologies currently used in industry. The Willowstick Technology was not used for this research. It is presented briefly herein for completeness. Further, future research using this technology is recommended.

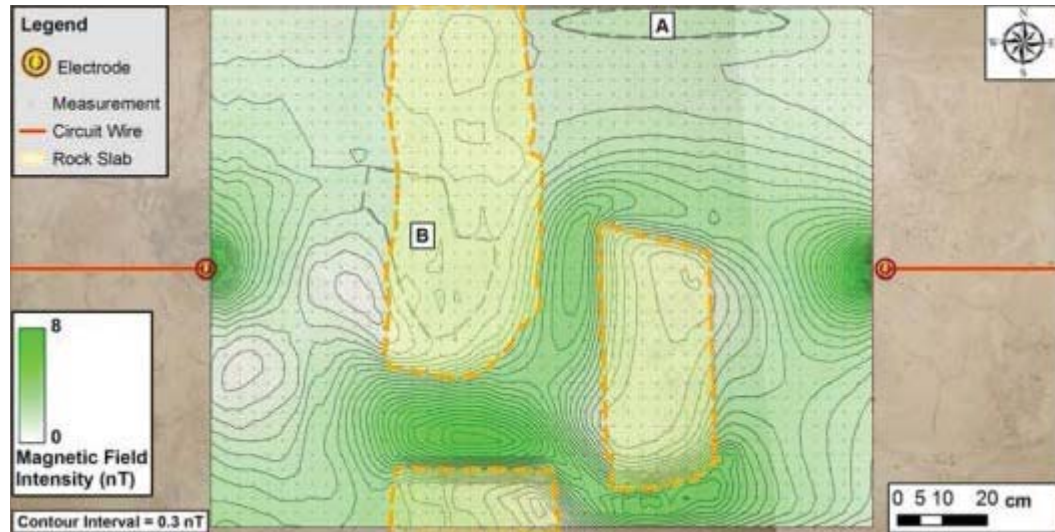


Figure 2.8: Magnetic field mapped using Willowstick technology (White Paper, 2012).

2.4 Associated Technology

As mentioned earlier in Chapter 1, the development of a new area performance parameter to assess the effectiveness of a grout curtain is essential to the success of this research. However, earlier studies (Bruce, D., 1982; Bruce, D., 1992; Uromeihy and Farrokhi, 2012; Gurocak and Alemdag, 2012) fail to use a performance parameter based on areas to assess the effectiveness of a grout curtain. These studies solely utilized Lugeon values to assess the effectiveness of a grout curtain. However, the studies conducted by Heinz Priebe in 1991 and 1995, utilized an area replacement ratio to determine an improvement factor which helps quantify the effectiveness with respect to ground improvement. Priebe (1991) and Priebe (1995) defined the area replacement ratio as the area of the improved ground divided by the total area of the unimproved ground. Equation 2 shows the area replacement ratio expression.

$$\text{Area replacement ratio} = \frac{A}{A_c} \quad (2)$$

where A = total area of the improved ground (area of the column(s)) and A_c = total area of the unimproved area.

The area replacement ratio is an expression that identifies the proportionality between the improved ground and unimproved ground. With further analysis, Priebe (1995) discovered a relationship that relates the area replacement ratio to an improvement factor (n) seen in Equation 3.

$$n = 1 + \left(\frac{A_c}{A} \right) \left(\frac{5 - A_c/A}{4(\tan^2(45 - \varphi_c/2))(1 - A_c/A)} - 1 \right) \quad (3)$$

where n = improvement factor, A = total area of the improved ground, A_c = total area of the unimproved area and φ_c = friction angle of the backfill material.

As can be seen by the relationship expressed in Equation 3, the ground improvement factor is a function of the reciprocal of the area replacement ratio and friction angle of the associated backfill material. Going a step further, Priebe discovered that the ground improvement factor was a function of poisson's ratio as well. Figure 2.9 illustrates the relationship from Equation 3 as a function of poisson's ratio.

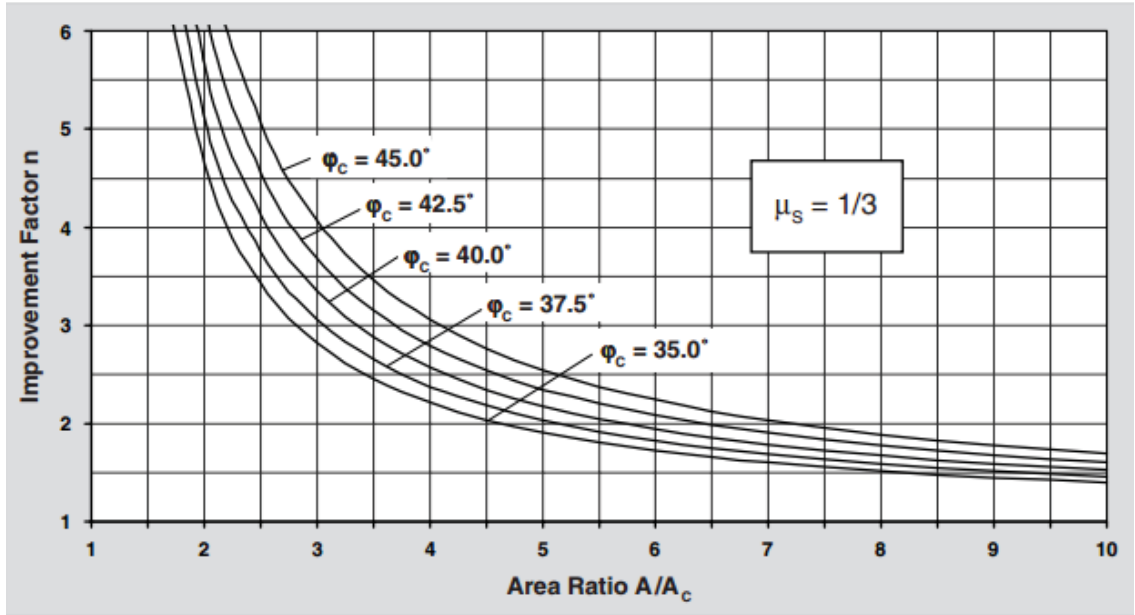


Figure 2.9: Relationship between area replacement ratio and ground improvement factor. (Priebe, 1995).

As seen in Figure 2.9, at a given area replacement ratio, friction angle and poisson's ratio, a ground improvement factor can be determined. This area replacement ratio provides a means to quantify the effectiveness of ground improvement. Little data has been presented using the area replacement ratio to evaluate the effectiveness of a grout curtain. However, it is believed that some type of area replacement ratio similar to the one Priebe (1991) and Priebe (1995) presented on ground improvement methods could be utilized in quantifying the effectiveness of a grout curtain.

2.5 Case History Studies

Four case history studies were investigated during this research. Case history data was obtained, to be used as the primary basis for which findings presented herein could be verified. Four separate dam structures were investigated in Hong et al. (2003)

research, which will later be used in thesis. The Hong et al. (2003) research investigated cut-off effects that are associated with rock grouting at various dam facilities using several parameters such as rock quality designation (RQD), injected cement volume, grout pressure and Lugeon values. The relationships that developed between these parameters were used in an attempt to develop a standard dam construction management program. Site properties, conditions and grouting properties can be seen in following table.

Table 2.1: Site conditions and grouting properties of four case history studies (Hong et al. 2003).

| | Site 1 | Site 2 | Site 3 | Site 4 |
|--------------------------------------|----------------------------------|--|----------------------------------|--|
| Type of Dam | Concrete Face Rock Fill Dam | Gravity Concrete Dam | Concrete Face Rock Fill Dam | Concrete Face Rock Fill Dam |
| Type of Rock | Metamorphic | Sedimentary | Sedimentary | Sedimentary |
| Number of Holes | 231 | 35 | 28 | 41 |
| Depth of Holes (m) | 40 | 20 | 20 | 40 |
| Grout Spacing | 1 column with 1.5 m spaced holes | 2 columns at 3.0 m zigzag interval; 3.0 m spaced holes | 1 column with 1.5 m spaced holes | 2 columns at 3.0 m zigzag interval; 3.0 m spaced holes |
| Span of Grout Curtain (m) | 347 | 105 | 42 | 123 |
| Average Lugeon Value Before Grouting | 3 | 3.9 | 11 | 2.37 |
| Average Lugeon Value After Grouting | 1 | 1.9 | 3 | 1 |
| Injection Pressure (Mpa) | 0.39-2.45 | 0.15-0.59 | 0.15-0.59 | 0.29-2.45 |
| Assumed Grout Column Diameter (m) | 1.016 | 1.016 | 1.016 | 1.016 |

However, the conclusions made in this research were predominately qualitatively based rather than quantitatively based. For research to be beneficial, relationships and behavior must be quantifiable. The research presented herein assessed the effectiveness of a grout curtain quantifiably.

2.6 Summary

As previously mentioned earlier in this thesis, monitoring Lugeon values during the installation of a grout curtain is the current state-of-practice when assessing the effectiveness of a grout curtain. It is believed that Lugeon values combined with a new performance parameter similar to the one presented by Priebe (1991) and Priebe (1995) could lead to a quantifiable relationship that helps predict the effectiveness of a grout curtain prior to installation. With very little data relating Lugeon values to a performance parameter that takes into account area, the need for further experimentation is proven necessary. However, this experimentation needs to be done using a laboratory-scale physical seepage model. The sheer amount of equipment needed along with the associated costs, deemed a field monitoring program impractical.

Chapter 3: Experimental Methods and Materials

3.1 Test Material – Index Testing and Material Characterization

All testing during the study was performed on two natural sands that were taken from sites located in the state of Kentucky. The first test sand used came from the banks of the Ohio River, near Newport, Kentucky. The second test sand was found along the banks of the Kentucky River directly North of Frankfort, Kentucky. The test sands herein will be referenced as Ohio River Valley sand and Kentucky River sand. The exact site locations where the Ohio River Valley sand and Kentucky River sand was found can be seen in Figure 3.1.

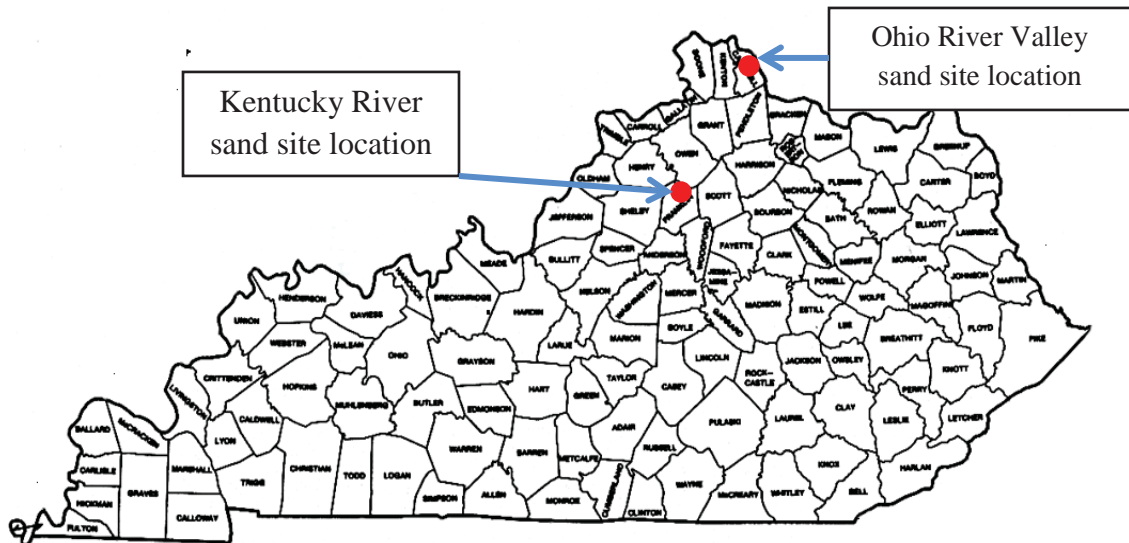


Figure 3.1: Site locations where the Kentucky River sand and Ohio River Valley sand were collected.

Initial index testing on the Ohio River Valley sand and Kentucky River sand consisted of four separate ASTM tests. The ASTM tests performed on the sand include

particle-size distribution of soils using sieve analysis (ASTM D6913), specific gravity (ASTM D854), maximum index unit weight using a vibratory table (ASTM D4253) and minimum index unit weight (ASTM D4254). Both test sands were a coarse, poorly graded sand (SP) according to the Unified Soil Classification System (USCS). However, the particle grain sizes of the Kentucky River sand were much finer than the Ohio River Valley sand. The test results from the various ASTM tests performed on the Ohio River Valley sand and Kentucky River sand are populated in Table 3.1. To view recorded data from the particle-size distribution sieve analysis and specific gravity test refer to Appendix A.

Table 3.1: Ohio River Valley sand and Kentucky River sand soil properties.

| Test Sand | Classification | Gs | D_{10} (mm) | D_{30} (mm) | D_{60} (mm) | Maximum Dry Unit Weight (kN / m^3) | Minimum Dry Unit Weight (kN / m^3) |
|---------------------------|----------------|------|------------------|------------------|------------------|--|--|
| Ohio River Valley Sand | SP | 2.65 | 0.30 | 0.50 | 0.81 | 19.08 | 15.27 |
| Kentucky River Sand | SP | 2.67 | 0.11 | 0.17 | 0.21 | 18.42 | 12.01 |

Where D_{10} = the diameter in the particle-size distribution curve corresponding to 10 percent finer, D_{30} = the diameter in the particle-size distribution curve corresponding to 30 percent finer, D_{60} = the diameter in the particle-size distribution curve corresponding to 60 percent finer and Gs = specific gravity.

3.2 Physical Model

A laboratory-scale physical seepage model was developed to aid in the understanding and development of geotechnical trends that evaluate the effectiveness of a grout curtain. All testing during this study was performed in a non-conductive test box. The inside width, length and height dimensions are 595-mm, 595-mm and 610-mm respectively. The box was constructed using polycarbonate for the sides, fiberglass angles and shims and an acrylic base (Huff, 2010). A silicon based product was used to prevent water from leaking through the cracks in between the polycarbonate siding and acrylic base. Figure 3.2 shows the configuration of the physical model from plan view.

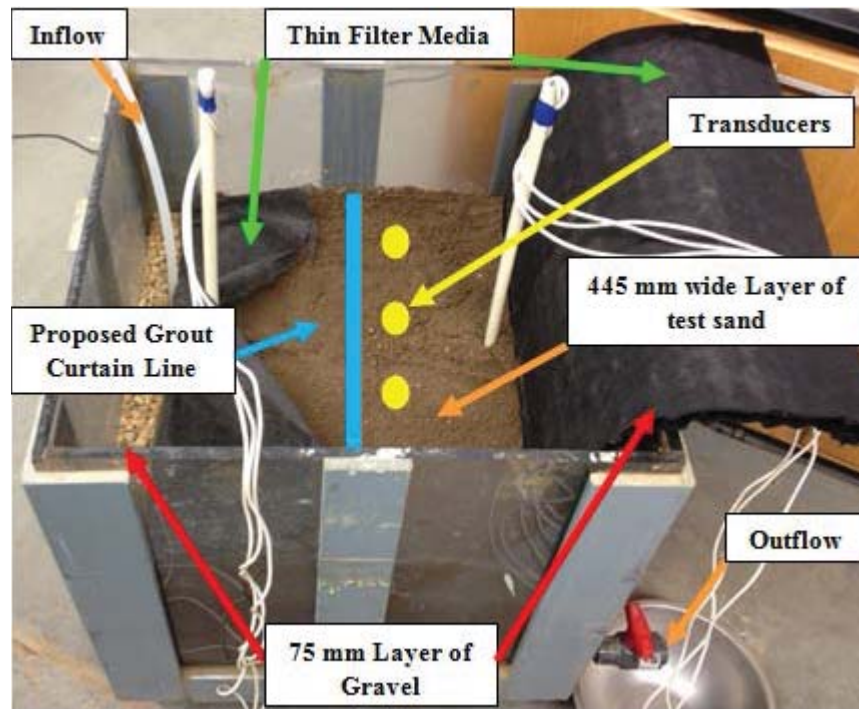


Figure 3.2: The composition of the working physical model.

The model consisted of upstream and downstream drainage media, with the test sand sandwiched in between. The drainage media consisted of vertical layers of gravel

that were approximately 75-mm thick. The test sand section was approximately 445-mm thick. A thin piece of geosynthetic fabric was used between the sand and the gravel on both the upstream and downstream side to prevent infiltration of fines into the gravel. Seepage was achieved by pumping water into the physical model on the upstream side and being drained on the downstream side.

To reach a desired test unit weight, the weight of oven dry test sand was measured. The sand was oven dried for 24 hours. The test sand was placed in three separate layers. Each layer was approximately 17.3-cm thick. Upon placement of each layer, the test sand was compacted. A 22.68 kg weight dropped from half a meter high onto a flat surface was used to reach the necessary test unit weights. A flat surface was used to allow for equal force distribution across the entire area of the test media. This allowed for a uniform compaction effort.

3.3 Pore Pressure Measurements

To measure various pore pressures within the physical model for this study, three miniature pore pressure transducers were utilized as piezometers and installed at pre-determined locations in the test box. In this study, pore pressure values were measured using Kulite XCL-11-250-150SG sealed gauge miniature pore pressure transducers connected to a National Instruments data acquisition system. Figure 3.3 shows three transducers connected to the National Instruments data acquisition system.



Figure 3.3: Three pore pressure transducers connected to the National Instruments data acquisition system.

LabVIEW 2012 software was utilized to acquire and analyze the data. The data acquisition system combined with the development of a program within the LabVIEW 2012 software, made it possible to convert electrical readings to their corresponding pore pressure measurements. To further understand the step-by-step process of creating the program, refer to Appendix B.

3.3.1 Pore Pressure Transducers

The specific miniature pore pressure transducer model utilized in this study of the evaluation of the effectiveness of a grout curtain was a XCL-11-250-150SG sealed gauge pore pressure transducer manufactured by Kulite which can be seen in Figure 3.4. Each individual pore pressure transducer had a fully active four arm Wheatstone Bridge. The

XCL-11-250-150SG miniature sealed gauge pore pressure transducers were rated at 1.03 MPa well within the ranges of pressures that are expected in the physical model.



Figure 3.4: Kulite's XCL-11-250-150SG sealed gauge pore pressure transducer model used in this study.

3.3.2 Pore Pressure Transducer Calibration

Self-verification of the calibration of the XCL-11-250-150SG pore pressure transducers was essential in producing reliable data for this study. According to Kulite, the XCL-11-250-150SG was calibrated in a water-filled pressure tank. To verify that the XCL-11-250-150SG transducers would provide reliable data for this study, the pore pressure transducers were placed at various heights in a acrylic cylindrical tube that was filled with saturated test sand. Several readings were taken at different known pressure increments to verify that the pore pressure readings coming from the pore pressure transducers were indeed accurate. Known pressure increments were determined by multiplying the unit weight of water by the height. Pore pressure measurements taken by the pore pressure transducers were compared to various known pore pressures to develop

a calibration equation. Pore pressure measurements were taken at the corresponding actual pressure readings of 0, 2.76, 4.14, 5.52 and 6.90 kPa. Figure 3.5 shows the calibration curves associated with the three pore pressure transducers used in this study.

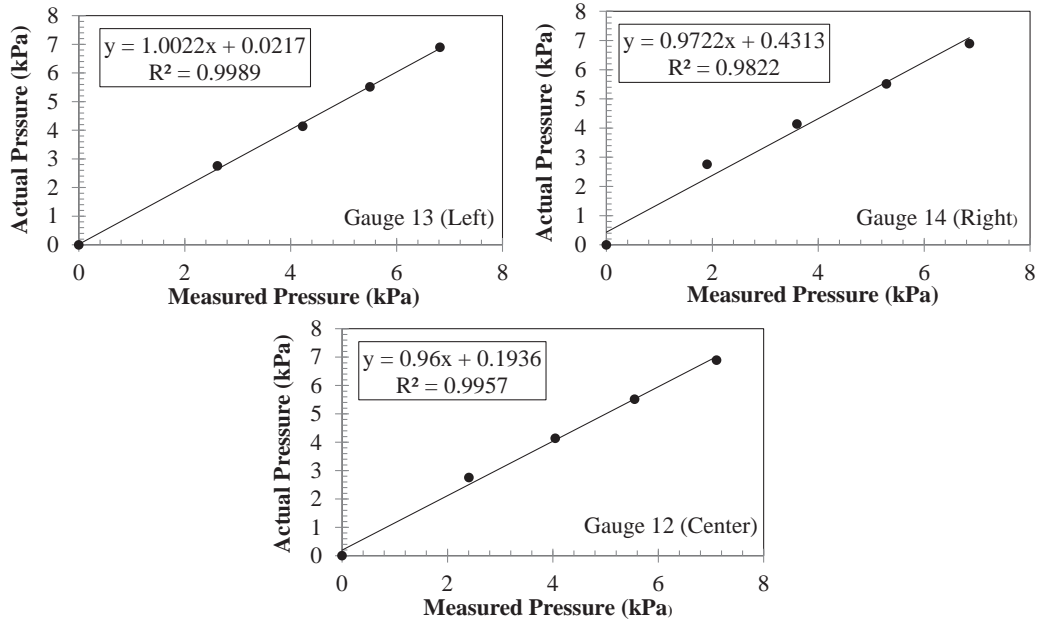


Figure 3.5: Calibration curve and equation associated with the three pore pressure transducers.

Each pore pressure transducer was calibrated individually, thus each pore pressure transducer has a corresponding calibration equation. The calibration equations used in this study are given as:

$$U_{ACT,13} = 1.022(U_{MEA,13}) + 0.0217 \quad (4)$$

$$U_{ACT,12} = 0.96(U_{MEA,12}) + 0.1936 \quad (5)$$

$$U_{ACT,14} = 0.9722(U_{MEA,14}) + 0.4313 \quad (6)$$

where U_{ACT} = actual pore pressure in kPa and U_{MEA} = measured pore pressure in kPa.

Manually calibrating the three pore pressure transducers assured that the measured pore pressures in the test box were in fact accurate.

3.4 Flow Rate Measurements

Along with pore pressure measurements, flow rate measurements were also utilized to evaluate the effectiveness of a grout curtain. Measuring flow rates is a common measurement that is taken when trying to assess the efficiency of a grout curtain. Flow rate is an easy parameter to measure since there is no need for extensive equipment. Also, measurements can be taken in timely fashion. Several studies (Bruce, D., 1982; Nappi, M. et al., 2005; Saeidi, O. et al., 2013) used flow rate measurements to help assist in determining the effectiveness of a grout curtain, similar to this study. Flow rate values were determined using an approach similar to a constant head test. The water level on the upstream side of the box was held constant, while being drained on the downstream side. The volume of water coming out of the box was captured and measured over a specified time interval. A flow rate measurement is calculated using the following equation:

$$q = \frac{Q}{t} \quad (7)$$

where q = flow rate, Q = total volume of water discharged and t = total time interval of test.

The flow rate measurements were used as an input parameter in the Houlby (1976) and Quinones-Rozo (2010) Lugeon value equation presented in Chapter 2, which helps in the quantification of the effectiveness of a grout curtain.

3.5 Lugeon Value Determination

Lugeon values were a key component to this study of quantifying the effectiveness of a grout curtain. Studies such as Hong (2003), Ajalloeian et al. (2012),

and Parrock et al (2010) used Lugeon values as the primary factor in determining the effectiveness of a grout curtain under a dam. With this being said, Lugeon values were incorporated into this study to not only be consistent with the current state-of-the-practice, but also to allow for model verification which will be discussed later in this thesis. Since the study was conducted in a laboratory-scale physical seepage model as opposed to out in the field, slight modifications in the determination of Lugeon values were taken. This research measured flow rates similar to a standard Lugeon test. The volume of water flowing through the porous test media was measured over a specified time interval. However, the type of pressure measurements utilized in this research varied slightly from the pressure measurements of a standard Lugeon test. A standard Lugeon test measures a water injection pressure (P), which is the pressure at which water is pumped through a porous media. However, the equipment available for this research was unable to measure a water injection pressure. Pore pressure measurements were utilized to help determine corresponding Lugeon values instead. Since this research did not measure water injection pressures directly, “true” Lugeon values were unable to be obtained. However, allowing sufficient time for pore pressure measurements to reach a state of equilibrium, allowed for pore pressure measurements to be used as water injection pressures. Although this research did not strictly measure “true” Lugeon values, it is still a reasonable approximation.

Chapter 4: Modeling the Grout Curtain

Two separate test procedures were used to model the grout curtains for this study. The first test procedure utilized flat acrylic slats to simulate grout columns while using Ohio River Valley sand as the test media. The second test procedure used polyvinyl chloride (PVC) pipes and Kentucky River sand. Flat acrylic slats were used in an attempt to quantify the effectiveness of a grout curtain based on lengths. PVC pipes were used in a similar fashion as the flat acrylic slats. However, PVC pipes were utilized to address the effectiveness of grout curtain based on areas. Using PVC pipes allowed for an accurate geometric representation of a true grout curtain. The test media changed between the two test procedures due to quantity issues. Both these sands classified as a poorly graded sand (SP) according to Unified Soil Classification System (USCS). However, comparing the D_{10} values for both of the test samples, it is evident that particle grain size of the Kentucky River sand is much finer than the particle grain size of the Ohio River Valley sand. Both procedures measure pore pressures and volume of discharge as subsequent simulated grout columns are placed in the test media in various orientations.

4.1 Linear Representation of Grout Curtain-Acrylic Slats Testing and Results

For this study, only one of three calibrated pore pressure transducers was installed at a pre-determined location within the soil profile. The pore pressure transducer was placed directly in the center of the test media. Pore pressure measurements along with corresponding volume discharge measurements were subsequently taken during the various installation sequences of the simulated grout curtain under a constant head. The volume discharge measurements were taken to calculate corresponding hydraulic

conductivity values based on constant head methodology. The combination of volume discharge measurements and several known dimensional parameters of the test media within the laboratory-scale physical seepage model allowed for the determination of hydraulic conductivity values based on the following equation.

$$K = \left(\frac{Q}{t} \right) \left(\frac{\Delta H}{L} \right) \left(\frac{1}{A} \right) \quad (8)$$

where K = hydraulic conductivity, Q = volume of water being discharged, t = time interval, ΔH = total head, L = length of the test media and A = cross-sectional area of test media.

Hydraulic conductivity values were determined in an attempt to identify different trends that could be used to help quantify the effectiveness of a grout curtain. Hydraulic conductivity values were believed to be a possible parameter to assess the effectiveness of a grout curtain similar to the research presented by Cotton and Matheson (1990).

4.1.1 Initial Equilibrium Testing

Before the grout curtain was placed, the test box was flooded by closing the drain and filling the box with water until the water level was 25-mm above the soil surface. After flooding the physical model with water, the drain was opened and the water was pumped into the physical model at a rate that kept the inflow rate equal to the outflow rate. In essence, the physical model was held at a constant head for 30 minutes before any initial testing was completed to provide adequate time for saturation. Running the initial test with no grout curtain present produced a hydraulic conductivity value of 0.0085 cm/s and a pore pressure value 3.17 kPa.

Another initial test that was conducted was a test that showed the change in pore pressure with respect to time with the presence of a simple grout curtain. The purpose of this test was to ascertain the amount of time required for pore pressures to reach equilibrium. The grout curtain consisted of two-acrylic slats, 76.2 mm-wide, inserted 25.4 mm upstream of the pore pressure transducer. Figure 4.1 shows the effects the simple grout curtain had on the pore pressure with respect to time. It also shows that approximately 20 minutes after the grout curtain was inserted 25.4 mm upstream of the pore pressure transducer, the pore pressure values began to level off and stay constant. This figure shows that pore pressure readings should be taken at least 20 minutes after the grout curtain is inserted into the Ohio River Valley sand to allow it to reach an equilibrium state.

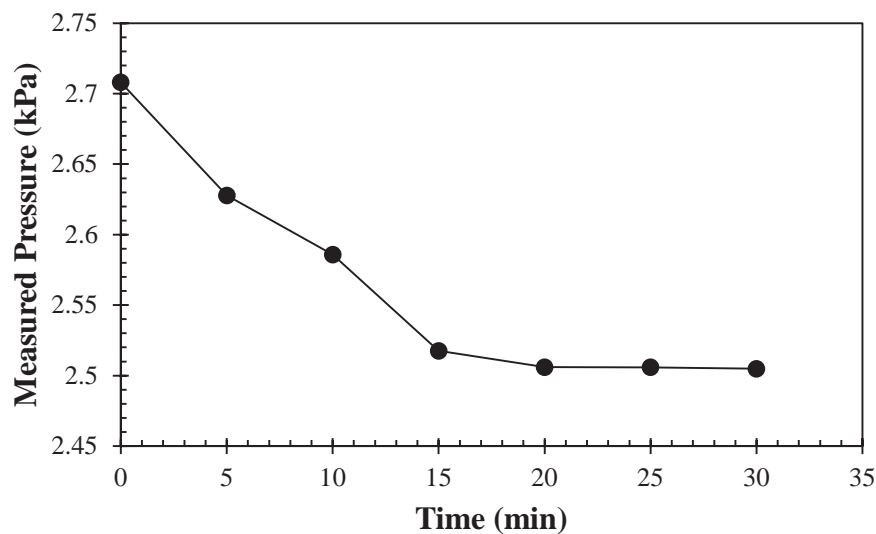


Figure 4.1: Effect of a simple grout curtain on pore pressure with respect to time.

It is important to note that the initial pressure at time zero in Figure 4.1 is not equal to the initial pressure without the presence of a simple grout curtain. Two separate tests were conducted using two different amounts of porous media.

4.1.2 Procedures Testing With Grout Curtain and Results

Experimentation was performed to ascertain the effect of installation sequence on the performance parameters. For this series of tests, pore pressure and hydraulic conductivity were used as performance parameters. The testing consisted of five sets of tests that used three orientation schemes. Each set of tests in this test series utilized two acrylic slats, 76.2-mm wide, inserted 25.4-mm upstream of the transducer. Each set of tests had its own grout curtain placement scheme along the proposed grout curtain line. The proposed grout curtain line within the physical model can be seen in Figure 4.2. In the figure, the numbers represent possible slat locations. Pore pressure and hydraulic conductivity values were measured during each test set.

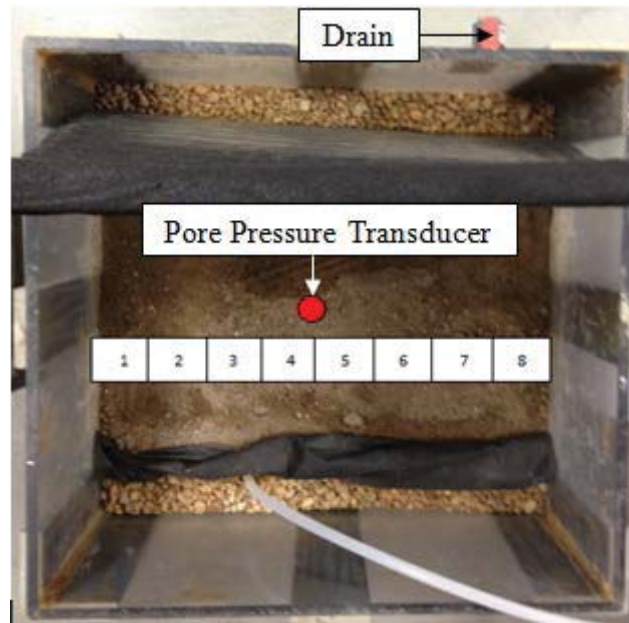


Figure 4.2: Eight possible slat locations along the proposed grout curtain line.

The first orientation scheme used in this test series was to move two 76.2-mm wide acrylic slats across the length of the proposed grout curtain line. Table 4.1 shows

the various grout orientations associated with Tests 1 through 7, along with their corresponding pore pressure and hydraulic conductivity measurements.

Table 4.1: Grout curtain orientations for Tests 1 through 7 along with their corresponding hydraulic conductivity and pore pressure measurements.

| Tests | Left Slat Location | Right Slat Location | Pore Pressure (kPa) | Hydraulic Conductivity (cm/s) |
|--------------|---------------------------|----------------------------|----------------------------|--------------------------------------|
| 1 | 4 | 5 | 3.09 | 0.0078 |
| 2 | 2 | 3 | 2.79 | 0.0081 |
| 3 | 1 | 2 | 2.40 | 0.0074 |
| 4 | 3 | 4 | 3.03 | 0.0071 |
| 5 | 5 | 6 | 2.82 | 0.0065 |
| 6 | 6 | 7 | 2.51 | 0.0069 |
| 7 | 7 | 8 | 2.32 | 0.0067 |

During Tests 1 through 4, the two-76.2 mm wide acrylic slats started directly in the middle of the box and moved to the left. However, during Tests 5, 6 and 7 the two-76.2 mm wide acrylic slats started directly in the middle of the box and moved to the right. By looking at Figure 4.3, the effects of the location of the drain can be seen. Tests 1 through 4, with the grout curtain on the left hand side, experienced higher pore pressures than Tests 5, 6 and 7, with the grout curtain on the right hand side, because the flow of water had a more direct path to the drain. Plotting pore pressure versus distance from the pore pressure transducer, Tests 1 through 4 and Tests 5, 6 and 7 both follow the same trend. As the two-76.2 mm wide acrylic slats get further and further away from the pore pressure transducer, the lower the pore pressure became. The assumption is that the lower the pore pressure, the more effective the grout curtain was.

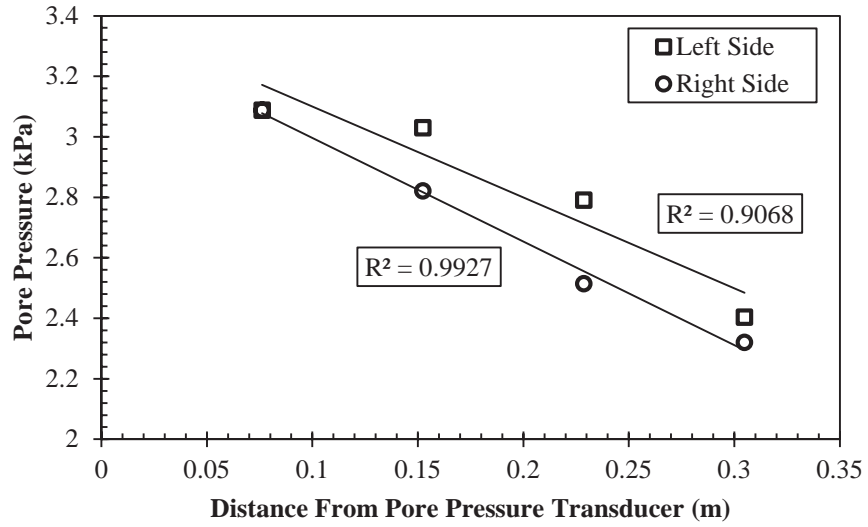


Figure 4.3: Pore pressure versus distance from pore pressure transducer for Tests 1-7.

The effect of the drain can also be seen in Figure 4.4 which plots hydraulic conductivity versus distance from the pore pressure transducer.

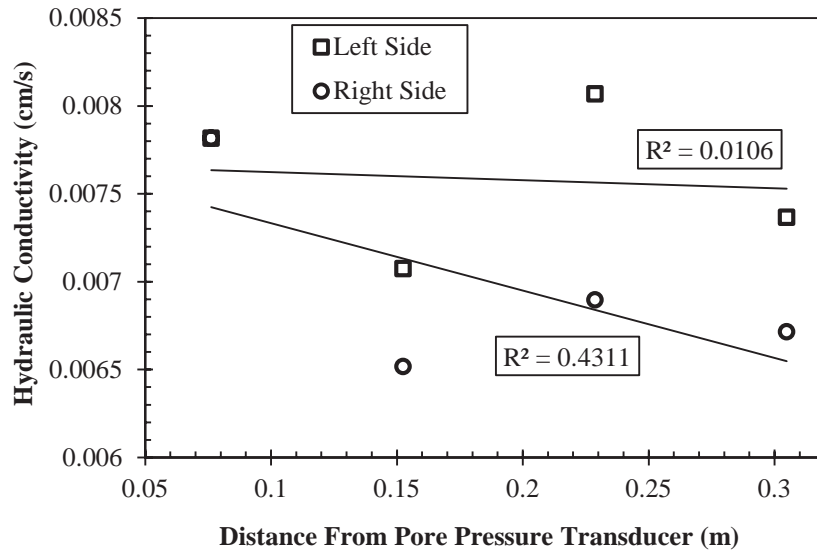


Figure 4.4: Hydraulic conductivity versus distance from pore pressure transducer for Tests 1 through 7.

Tests 1 through 4 with the grout curtain on the left hand side, experienced higher hydraulic conductivity values than Tests 1, 5, 6 and 7 with the grout curtain on the right hand side because the flow of water had a more direct path to the drain. Other than the observed behavior previously discussed, no further trends developed with respect to hydraulic conductivity in this study.

The second orientation scheme used in this study started out with one-76.2 mm wide acrylic slat on each end of the proposed grout curtain line. The slats then preceded inwards towards the pore pressure transducer. This orientation scheme was conducted to help determine whether or not changes in sequencing had any kind of effect on pore pressure and hydraulic conductivity. Table 4.2 shows the various grout curtain orientations for Tests 1, 8, 9 and 10 along with their corresponding pore pressure and hydraulic conductivity measurements. For further clarity on this specific grout curtain orientation scheme, refer to Table 4.2 and Figure 4.2.

Table 4.2: Grout curtain orientations for Tests 1, 8, 9 and 10 along with their corresponding hydraulic conductivity and pore pressure measurements.

| Tests | Left Slat Location | Right Slat Location | Pore Pressure (kPa) | Hydraulic Conductivity (cm/s) |
|--------------|--------------------|---------------------|---------------------|-------------------------------|
| 8 | 1 | 8 | 2.41 | 0.0065 |
| 9 | 2 | 7 | 2.63 | 0.0065 |
| 10 | 3 | 6 | 3.10 | 0.0063 |
| 1 | 4 | 5 | 3.09 | 0.0078 |

The results obtained from this series of tests, showed that sequencing does have an effect on pore pressure and hydraulic conductivity. As the acrylic slats propagated towards the

pore pressure transducer, higher pore pressures resulted. Hydraulic conductivity stayed fairly constant as the acrylic slats preceded inward, with the exception of Test 1.

The third orientation scheme used in this study was holding one-76.2 mm-wide acrylic slat stationary in the middle next to the pore pressure transducer, while moving the other 76.2 mm wide acrylic slat outward along the proposed grout curtain line. This orientation scheme was conducted to help determine the effects on pore pressure and hydraulic conductivity with respect to which side the simulated grout curtain was placed. Table 4.3 shows the various grout curtain orientations for Tests 11-16 along with their corresponding pore pressure and hydraulic conductivity measurements.

Table 4.3: Grout curtain orientations for Tests 11 through 16 along with their corresponding hydraulic conductivity and pore pressure measurements.

| Tests | Left Slat Location | Right Slat Location | Pore Pressure (kPa) | Hydraulic Conductivity (cm/s) |
|--------------|---------------------------|----------------------------|----------------------------|--------------------------------------|
| 11 | 4 | 8 | 2.30 | 0.0064 |
| 12 | 4 | 7 | 2.69 | 0.0067 |
| 13 | 4 | 6 | 3.04 | 0.0071 |
| 14 | 1 | 5 | 2.39 | 0.0064 |
| 15 | 2 | 5 | 2.78 | 0.0067 |
| 16 | 3 | 5 | 3.13 | 0.0071 |

Test results associated with this test series showed that when the simple grout curtain was placed on the right hand side of box along the proposed grout curtain line, pore pressures were lower compared to when grout curtain was on the left hand side. Upon speculation it was believed that the observed pore pressure behavior was a result of the water flow path. Since the water had a more direct path to the drain when the simulated grout curtain was

positioned on the left hand side, higher pore pressures resulted as compared to when the grout curtain was positioned on the right hand side. Hydraulic conductivity stayed constant, when comparing measurements with the grout curtain on the left and right hand sides.

4.2 Development of a New Performance Parameter–Linear Replacement Ratio

As was discussed earlier in Chapter 2, it was hypothesized that an area replacement ratio could be used to quantify effectiveness. However, the use of flat acrylic slats precluded the use of areas. Therefore, a linear replacement ratio was developed. The linear replacement ratio was equal to the width of the installed slats divided by the total width of the model. Unfortunately, this relationship was inadequate because it did not take into account the various installation sequences. In addition, initial testing showed the location of the pore pressure transducer and drain influenced the results.

To evaluate the effectiveness of a grout curtain, this study utilized a linear replacement ratio (\bar{x}_2) based on a double normalization scheme. The linear replacement ratio takes into account the distance the centroids of the acrylic slats are from the pore pressure transducer and drain, respectively. This normalization scheme was conducted to understand the influence of the drain within the laboratory-scale physical model. The linear replacement ratio equation that was developed is given in Equation 9 and is graphically presented in Figure 4.5

$$\bar{x}_2 = \frac{\left(\frac{d_1 + d_2}{L_1 + L_2} \right)}{\left(\frac{d_1 + d_2}{D_1 + D_2} \right)} \quad (9)$$

where \bar{x}_2 = double normalization value, d_1 = the distance between the centroid of the first slat and the midpoint of the pore pressure transducer, d_2 = the distance between the centroid of the second slat and the midpoint of the pore pressure transducer, L_1 = the length of the first slat, L_2 = the length of the second slat, D_1 = the distance from the centroid of the first slat to the midpoint of the drain, and D_2 = the distance from the centroid of the second slat to the midpoint of the drain.

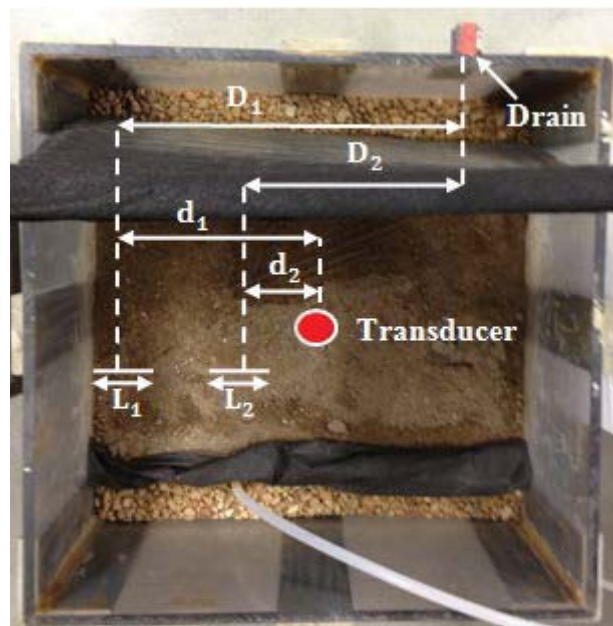


Figure 4.5: Graphical representation of the linear replacement relationship.

Figure 4.6 presents a plot of the linear replacement ratio versus pore pressure for all tests in this study. Refer back to Table 4.1, Table, 4.2 and Table 4.3 for further clarity on grout curtain orientations for all tests in this study. It can be seen in Figure 4.6 that the data produced well-defined curves. Figure 4.6 shows as the total length of the slats got further away from the pore pressure transducer the pore pressure decreased. The lower the pore pressure, the more effective was the grout curtain.

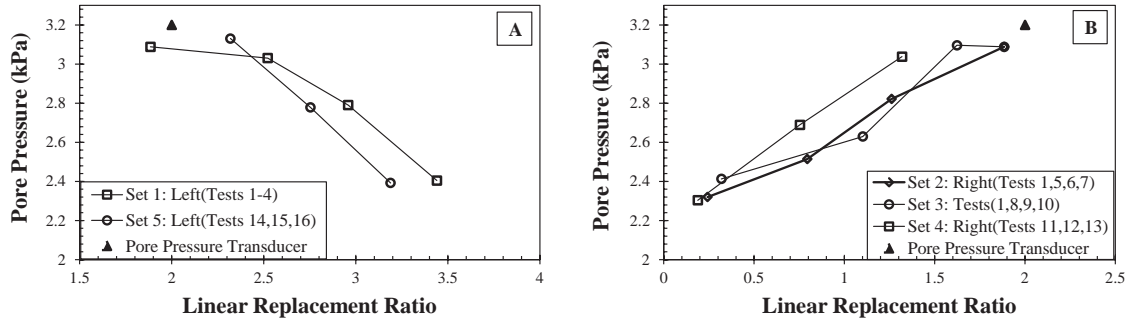


Figure 4.6: Linear replacement ratio versus pore pressure graph with grout curtain on (A) left hand side, and (B) right hand side.

The effects of the positioning of the drain within the laboratory-scale physical seepage model can be seen as well. Comparing tests with the same sequence, but on opposite sides of the physical model show the effects the drain position has on pore pressure measurements. When the grout curtain was placed on the left hand side of the physical model, pore pressure measurements tended to be higher than when the grout curtain was placed on the right hand side. This trend was consistent for all tests completed in this study since the flow of water had a more direct path to the drain when the grout curtain was on the left hand side of the physical model. The linear replacement ratio coupled with pore pressure measurements produced trends that will help contribute to quantification of the effectiveness of a grout curtain. However, the use of flat acrylic slats to represent a grout curtain wall does not encapsulate the true geometry of a grout curtain wall. Thus, the study used polyvinyl chloride (PVC) piping to take into account area as opposed to just length.

4.3 Area Representation of Grout Curtain-Polyvinyl Chloride Pipe Testing and Results

Similar to the acrylic slats testing, pore pressure transducers were utilized and installed at a pre-determined location within the test media. However, it is important to note that three pore pressure transducers were utilized, the test media used was the Kentucky River sand and the simulated grout columns were PVC pipes. Just like the acrylic slats testing, pore pressure measurements along with volume discharge measurements were subsequently performed during the various installation sequences of the simulated grout curtain. Measurements were taken every 25 minutes after the placement of PVC piping to allow for pore pressures to reach an equilibrium state. This time interval was determined from monitoring pore pressures and times in the manner presented in the “Initial Equilibrium Testing” section. However, it is noted that the time intervals were slightly different. Testing with the Kentucky River sand required measurements to be taken at 25 minutes instead of 20 minutes. The slight difference in time interval is due to a lower hydraulic conductivity.

Twenty sets of tests were performed during this test series. The twenty sets of tests were comprised of five spacing-to-diameter ratios (S/D) at four different dry unit weights. The spacing-to-diameter ratio is the ratio of the center-to-center spacing between grout columns along the proposed grout curtain line to the diameter of grout column itself. The diameter of all grout columns was 22 mm. The five different S/D ratios used in this study include 1, 1.18, 1.45, 1.62 and 2.17. These S/D ratios were selected to accommodate for equal spacing between grout columns. Not only did these S/D ratios accommodate for equal spacing, but they also allowed for the two grout columns at the ends of the proposed grout curtain line to touch the sides of the test box. The four dry

unit weights that were tested were 12.56, 13.03, 13.35, and 13.82 kN/m³. The dry unit weights were selected to fit within the maximum and minimum relative unit weights tabulated earlier in Table 3.1. But before PVC testing was performed, it was essential for this research that a new performance parameter utilizing areas be developed. By using the development of a new performance parameter combined with measured Lugeon values, it was believed that a grout curtain's effectiveness could be quantified prior to installation.

4.4 Development of a New Performance Parameter – Area Replacement Index

As mentioned earlier in Chapter 2, it was believed that an area replacement ratio similar to the one presented by Priebe (1991) and Priebe (1995) for ground improvement could be utilized to help in the quantification of the effectiveness of a grout curtain. Through experimentation with various proposed area replacement approaches, a new performance parameter was developed, which will be referenced throughout the remainder of this thesis as the area replacement index (λ).

To evaluate the effectiveness of a grout curtain, this study utilized a area replacement index value based on a double normalization scheme. The double normalization scheme was developed to take into account the amount of grout curtain placed within the area of improved ground and geometric configuration. The development of the area replacement index was based on four separate geometric configurations which are illustrated in Figure 4.7. Pore pressure measurements were subsequently taken as each additional grout column was installed following one of the geometric configurations.

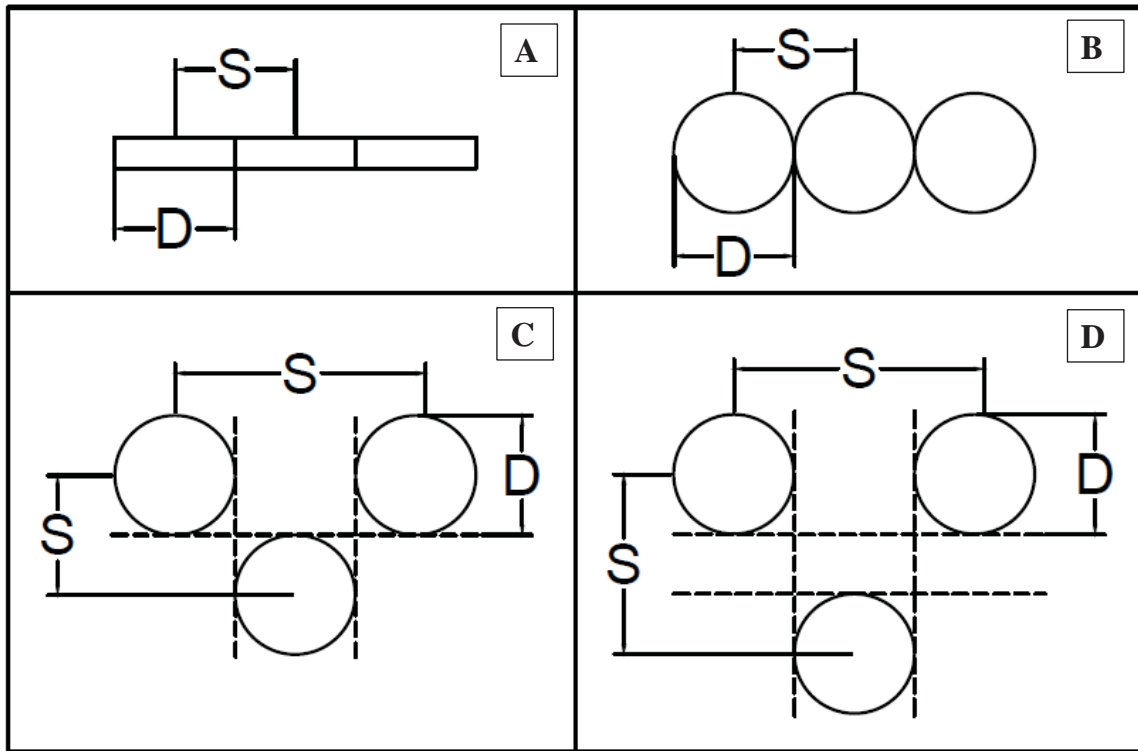


Figure 4.7: Geometric configurations used in the development of the area replacement index. (A) Tangent Planer, (B) Tangent Circular, (C) Tangent Primary-Secondary and (D) Discontinuous Primary-Secondary.

Seen in Figure 4.7, the tangent planer configuration is a rectangular trench section similar to a slurry wall panel. Testing was completed using flat acrylic slats, similar to the ones discussed earlier in this chapter. Tangent circular configuration is composed of a single row of circular grout columns placed tangent to one another. Both, tangent primary-secondary and discontinuous primary-secondary configurations are composed of two rows of circular grout columns. The only difference is the second row of grout columns for the discontinuous primary-secondary configuration is offset by a distance of half the diameter of a grout column. It is important to note that configurations identified

in Figure 4.7 as B, C and D utilized the same 2.2-cm diameter PVC piping for grout columns.

The area replacement index utilizes a double normalization scheme. The first normalization scheme utilized is illustrated by the following:

$$\bar{A}_G = \left(\frac{\sum^i A_{GC}}{A_{TOTAL}} \right) \quad (10)$$

where \bar{A}_G = the normalized grouted area, A_{GC} = summation of the area of the grout columns, i = the number of grout columns installed at a specific time, A_{TOTAL} = total area of ground to be improved given the width of grout column by the total length of the grout region. For further clarification, Figure 4.8 shows a generalized graphical representation of the proposed grout curtain region and components.

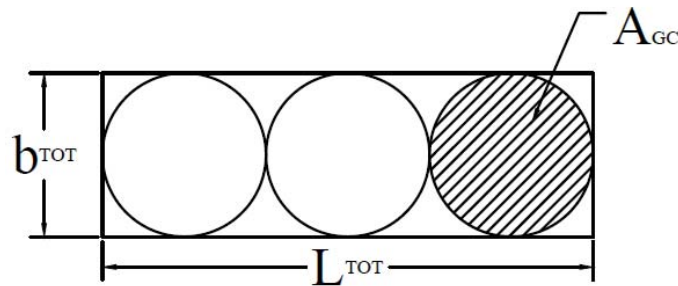


Figure 4.8: Components of grout region where $A_{TOTAL} = (b_{TOT})(L_{TOT})$, b_{TOT} = width of the grout region and L_{TOT} = length of the grout region.

Normalizing A_{GC} by A_{TOTAL} , allows for the evaluation of the effects of the area with respect to the area of improved ground. The relationship between normalized grouted area versus pore pressure for the four geometric configurations are shown in Figure 4.9.

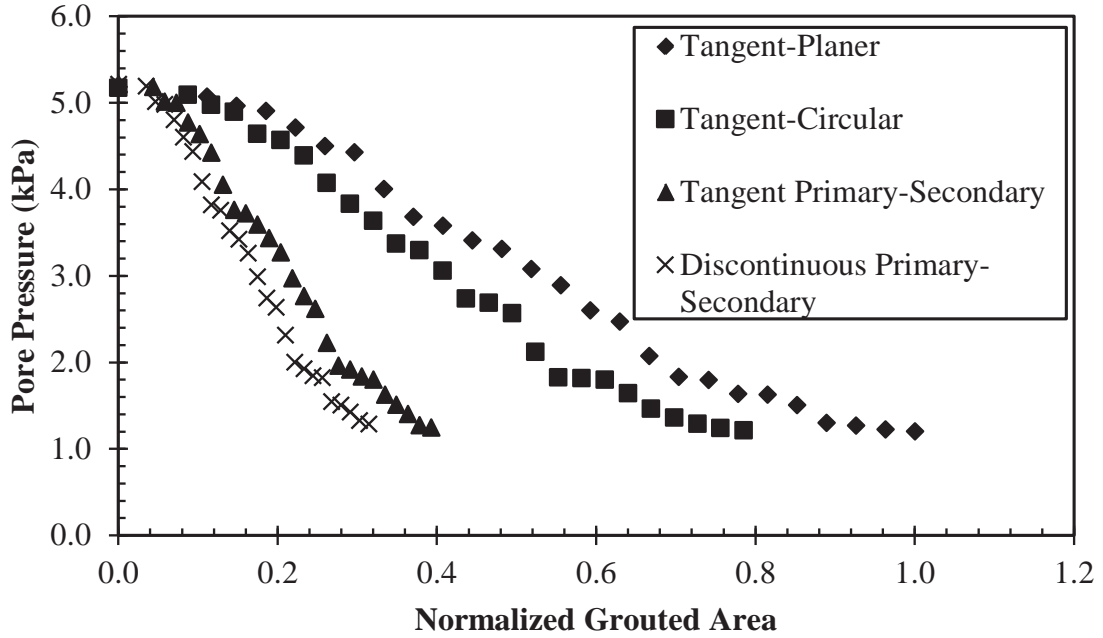


Figure 4.9: Relationship between normalized grouted area versus pore pressure for various geometric configurations.

As seen in Figure 4.9, in addition to varying with the normalized grouted area, pore pressure also varied consistently with respect to the geometric configuration. Thus, further normalization was done to take into account geometric configuration. The second normalization scheme takes into account geometric configuration and is given in the following form:

$$\lambda = \left[\frac{\left(\sum_1^i A_{GC} / A_{TOTAL} \right)}{\left(A_{GC(MAX)} / A_{TOTAL} \right)} \right] \quad (11)$$

where λ = area replacement index, $A_{GC(MAX)}$ = the maximum area of the grout columns that will fit within the grouted region at a specific S/D ratio.

Normalizing Equation 11 by $A_{GC(MAX)}/A_{TOTAL}$ takes into consideration geometric configuration. For example, if the geometric cross-section of the grout column is anything other than circular, this ratio will take this into consideration. Figure 4.10 shows the relationship between area replacement index versus pore pressure grouped by geometric configuration.

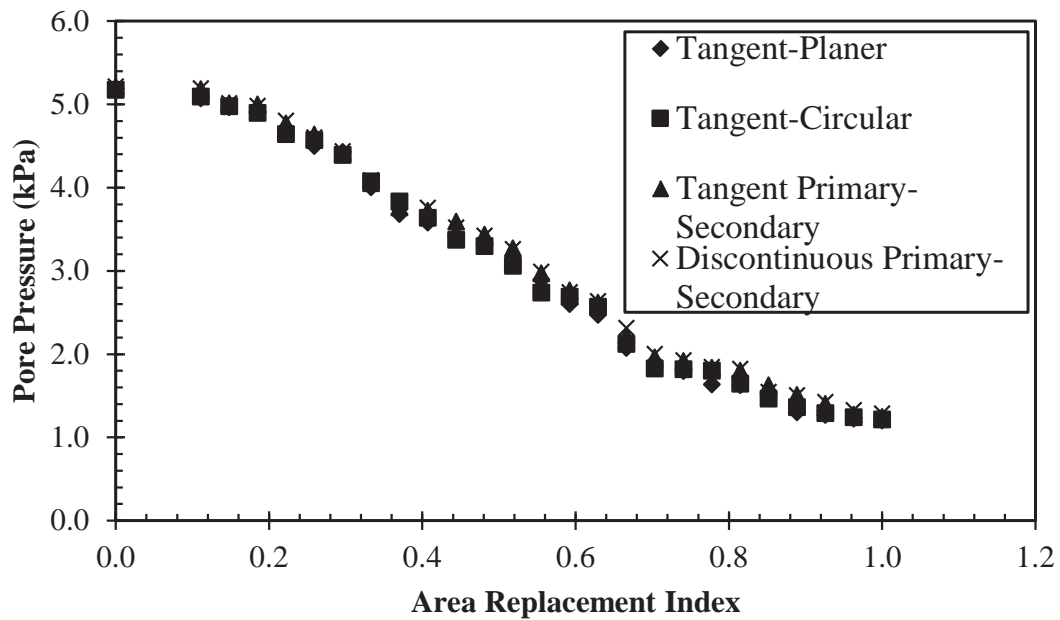


Figure 4.10: Relationship between area replacement index versus pore pressure grouped by geometric configuration.

As seen in Figure 4.10, the curves presented in Figure 4.9, collapsed to one curve after further normalization. At a constant area replacement index, pore pressure measurements at different geometric configurations seem to be equal. This is an indication that the area replacement index takes into consideration the amount of grout curtain placed within the area of improved ground and the geometric configuration. Thus,

to simply testing procedures, all further testing was completed using the tangent circular geometric configuration only.

4.5 Nested Pore Pressure Transducers

Nested pore pressure testing with PVC pipes was similar to the acrylic slats testing. However, it is important to note that the test media used was the Kentucky River sand, the simulated grout columns were PVC pipes with a diameter of 22 mm, and three pore pressure transducers were utilized and positioned differently than when performing the acrylic slats testing. The pore pressure transducers were situated in a manner within the porous media, where all three transducers were within the same column at various known pressures, hence the word nested. The three pore pressure transducers were nested directly in the center of the box. Gauge 12, Gauge 13 and Gauge 14 were placed in a vertical column at known reference pressures of 2.29 kPa, 3.62 kPa and 5.17 kPa, respectively. Known reference pressures were determined by multiplying the unit weight of water by the height. This series of tests was performed to determine a pore pressure gradient with respect to depth. Having this data would help illustrate the effects on pore pressure with respect to depth at different S/D ratios and dry unit weights.

Pore pressure and volume discharge measurements were taken, as each additional PVC pipe was placed along the proposed grout curtain line within the test box. Upon completion of the initial test, three-22 mm diameter PVC pipes were placed into the center of the test box along the proposed grout curtain line at the desired S/D ratios. Pore pressure and volume discharge measurements were again taken 25 minutes after the placement of PVC pipes. With these two measurements, corresponding Lugeon values

were calculated by utilizing Equation 1 presented in Chapter 2. This process was repeated adding one additional PVC pipe at the same desired S/D ratio, while alternating sides of the proposed grout curtain line until the grout curtain reached the sides of the test box. The entire process was repeated for all tests. Table 4.4 shows raw data corresponding to the test where the S/D ratio = 1 and a dry unit weight = 12.56 kN/m³. For additional test data corresponding to nested pore pressure transducer testing see Appendix C.

Table 4.4: Raw data from corresponding to S/D=1 and dry unit weight= 12.56 kN/m³.

| Test | # of PVC Pipes | Pore Pressure-Gauge 12(kPa) Top | Pore Pressure-Gauge 13(kPa) Middle | Pore Pressure-Gauge 14(kPa) Bottom | Discharge Volume (mL) |
|--------|----------------|---------------------------------|------------------------------------|------------------------------------|-----------------------|
| Run 0 | 0 | 2.352 | 3.690 | 5.159 | 5520 |
| Run 1 | 3 | 2.101 | 3.390 | 5.098 | 5423 |
| Run 2 | 4 | 2.167 | 3.515 | 5.075 | 5215 |
| Run 3 | 5 | 1.937 | 3.287 | 4.952 | 5193 |
| Run 4 | 6 | 1.841 | 3.127 | 4.683 | 4956 |
| Run 5 | 7 | 1.840 | 3.222 | 4.652 | 4720 |
| Run 6 | 8 | 1.831 | 3.262 | 4.503 | 4631 |
| Run 7 | 9 | 1.512 | 3.087 | 4.213 | 4321 |
| Run 8 | 10 | 1.343 | 3.058 | 4.243 | 3812 |
| Run 9 | 11 | 0.905 | 2.335 | 3.975 | 3561 |
| Run 10 | 12 | 0.425 | 1.857 | 3.721 | 3389 |
| Run 11 | 13 | Dry | 1.471 | 3.521 | 3251 |
| Run 12 | 14 | Dry | 1.052 | 3.275 | 3000 |
| Run 13 | 15 | Dry | 0.845 | 2.875 | 2651 |
| Run 14 | 16 | Dry | 0.642 | 2.683 | 2563 |
| Run 15 | 17 | Dry | 0.411 | 2.596 | 2312 |
| Run 16 | 18 | Dry | 0.213 | 2.318 | 2015 |
| Run 17 | 19 | Dry | Dry | 2.136 | 1875 |
| Run 18 | 20 | Dry | Dry | 2.006 | 1623 |
| Run 19 | 21 | Dry | Dry | 1.945 | 1452 |
| Run 20 | 22 | Dry | Dry | 1.842 | 1261 |
| Run 21 | 23 | Dry | Dry | 1.812 | 1205 |
| Run 22 | 24 | Dry | Dry | 1.625 | 1195 |
| Run 23 | 25 | Dry | Dry | 1.425 | 1182 |
| Run 24 | 26 | Dry | Dry | 1.358 | 1099 |
| Run 25 | 27 | Dry | Dry | 1.309 | 1082 |

It is observed that Gauge's 12 and 13 were unable to record pore pressures for all tests. Gauge 12 and Gauge 13 were able to record data until Run 10 and Run 16, respectively. Two of the pore pressure transducers read "Dry" because the pore pressure transducers were located above the ground water table. This same problem was encountered for several tests at different S/D ratios and dry unit weights. Aware that this would present a problem, it was determined that pore pressure transducers be placed at the bottom of the test box as the research moved forward. Placing the pore pressure transducers at the bottom of the box prevented them from reading "Dry".

4.6 Pore Pressure Transducers Located at Bottom of Test Box

Due to pore pressure transducers drying out, it was determined that the three pore pressure transducers be placed at the bottom of the test box in a line parallel to the proposed grout curtain seen in Figure 3.2. They were placed 25.4-mm downstream of the proposed grout curtain, equal distant from one another. The test procedure outlined in the "Nested Pore Pressure Transducers" section was followed. The only difference was the placement of the three pore pressure transducers. Gauge 12, 13, and 14 were placed in the center, left and right hand sides of the box looking downstream, respectively. The pore pressure transducers were placed in such a fashion in hopes to see if there were any influences from the drain. However, it was determined through plotting pore pressure for all three transducers versus area replacement index, the drain had little influence on the readings of the pore pressure transducers. Figure 4.11 illustrates the influence of the drain from one test set ($S/D = 1$ and dry unit weight = 12.56 kN/m^3).

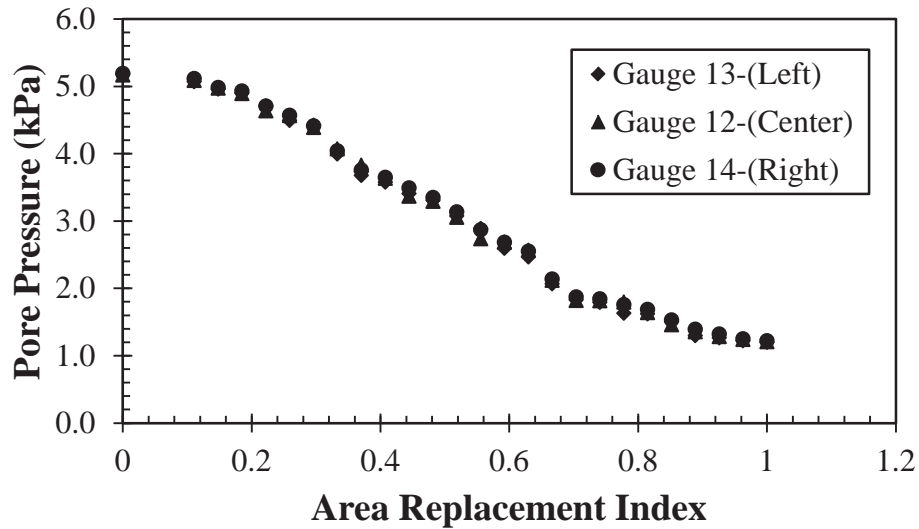


Figure 4.11: Influence of the drain on the three pore pressure transducers.

As observed in Figure 4.11, there was no significant variance in the pore pressure readings between the three transducers. Since this was consistent across all 20 tests, it was determined that an average pore pressure from the three transducers be used for further analysis.

Once test data was collected for all 20 test sets (five S/D ratios at four various unit weights), the data was analyzed for the intent of developing a quantifiable relationship between the area replacement index, Lugeon value, S/D ratio, and dry unit weight. To do so, various plots were developed to help in the understanding. The area replacement index was plotted versus several combinations and variations of parameters. It was determined through experimentation that combinations of average pore pressure, Lugeon values, and flow loss would provide the most correlation.

Flow loss is a typical parameter measured when calculating Lugeon values and can be seen in Equation 12:

$$Flow\ Loss = \frac{q}{L} \quad (12)$$

where q = flow rate out of the test box = Q/t and Q = volume of discharge, t = total time interval with which volume of discharge was collected and L = test interval length of the representative test sample.

In order to scale the results from the test box, average pore pressure, flow loss and Lugeon values were normalized as:

$$Normalized\ Average\ Pore\ Pressure = \frac{P}{P_0} \quad (13)$$

$$Normalized\ Flow\ Loss = \left[\frac{\left(\frac{q}{L} \right)}{\left(\frac{q_0}{L_0} \right)} \right] \quad (14)$$

$$Normalized\ Lugeon\ Value = \frac{LV}{LV_0} \quad (15)$$

where P_0 , q_0/L_0 , LV_0 = initial test measurements with no PVC pipes.

The normalization factors utilized in this research were from the initial test measurements described earlier in this chapter, when no PVC pipes were present, just the test media itself. Plots using these parameters are shown throughout the remainder of this chapter.

Figure 4.12 shows the relationship between area replacement index and normalized average pore pressure and shows how that relationship varies as the S/D ratio varies.

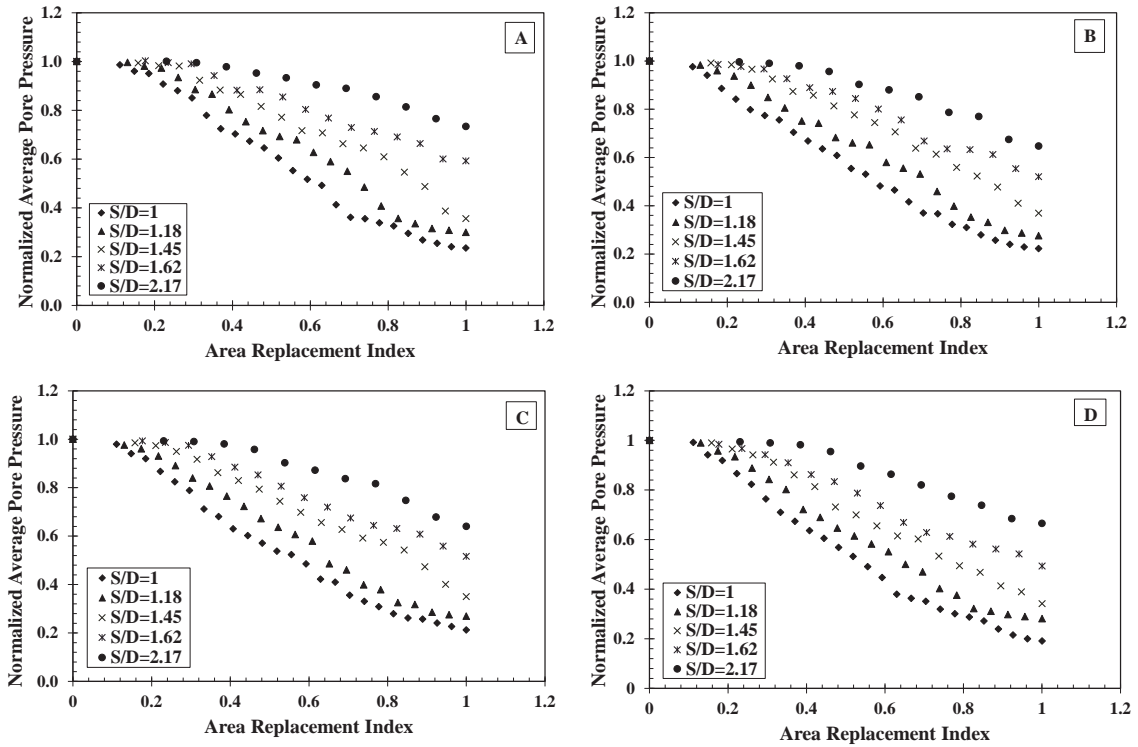


Figure 4.12: Area replacement index versus normalized average pore pressure, grouped by S/D ratios for a dry unit weight of (A) 12.56 kN/m³, (B) 13.03 kN/m³, (C) 13.35 kN/m³ and (D) 13.82 kN/m³.

For all 20 tests, as the area replacement index increases, the normalized average pore pressure decreases. Furthermore, at a constant area replacement index, the S/D ratio has a large effect on the normalized average pore pressure. For a given area replacement index, the relationship between the change in normalized average pore pressure and the change in S/D ratio do not appear to be proportional. However, there does seem to be some correlation between the shapes of the curves. All tests performed, display a similarly shaped curve.

Figure 4.13 illustrates the relationship between area replacement index and normalized q/L and shows how that relationship varies as the S/D ratio varies.

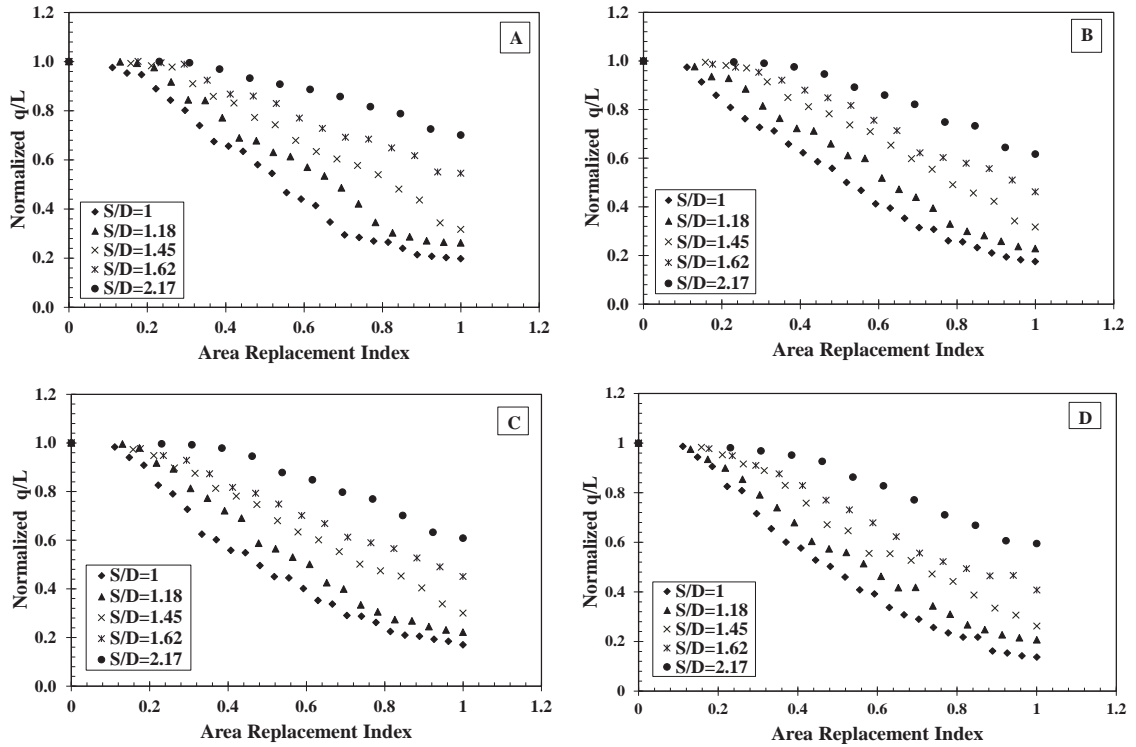


Figure 4.13: Area replacement index versus normalized q/L , grouped by S/D ratios for a dry unit weight of (A) 12.56 kN/m³, (B) 13.03 kN/m³, (C) 13.35 kN/m³ and (D) 13.82 kN/m³.

At a constant area replacement index, the S/D ratio has an effect on normalized q/L seen in Figure 4.13. Also, at a constant S/D ratio, as the area replacement index increases, the normalized q/L decreases. Looking at Figure 4.12 and Figure 4.13, the relationship between area replacement index and normalized q/L and the relationship between area replacement index and normalized average pore pressures are quite similar. The behavior of these curves presented seem to be consistent across all S/D ratios and dry unit weights.

The curves presented in Figure 4.12 and Figure 4.13, were curve fitted using a fitting program, Table Curve (SYSTAT, Chicago, IL). The curves presented in these figures, were curve fit to identify the general equation form that describes the behavior of these curves. Figure 4.14 illustrates fitted curves showing the relationship between area replacement index and normalized average pore pressure, and area replacement index versus normalized q/L at a dry unit weight of 12.56 kN/m^3 . It should be noted that similar behavior was witnessed across all dry unit weights. However, for simplicity only results for a dry unit weight of 12.56 kN/m^3 were presented.

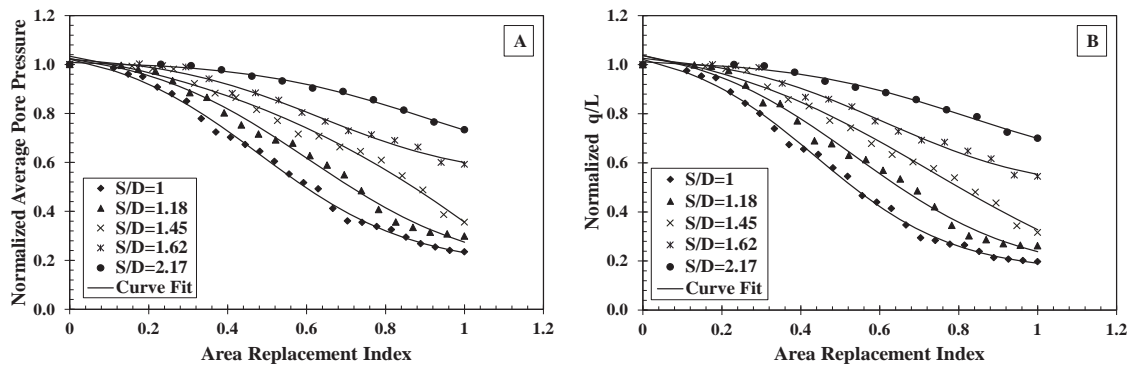


Figure 4.14: (A) Fitted curves showing the relationship between area replacement index versus normalized average pore pressure, grouped by S/D ratios for a dry unit weight of 12.56 kN/m^3 . (B) Fitted curves showing the relationship between area replacement index versus normalized q/L , grouped by S/D ratios for a dry unit weight of 12.56 kN/m^3 .

The general equation describing the behavior of the curves presented in Figure 4.14 were identified as a sigmoid. The specific equation that was identified having the highest R^2 values across all S/D ratios and dry unit weights can be seen in following equation:

$$y = a + \left(\frac{b}{1 + \exp\left(-\left(\frac{x-c}{d}\right)\right)} \right) \quad (16)$$

where y = normalized average pore pressure or normalized q/L , x = area replacement index, and a, b, c, d = constants.

Constants a, b, c and d were unable to be identified as specific properties of the curve through further analysis. However, for completeness constants a, b, c, d and R^2 values corresponding to the curve-fit plots of area replacement index versus normalized average pore pressure, and area replacement index versus normalized q/L are tabulated in Table 4.5. The constants and R^2 values are tabulated for all four dry unit weights.

Table 4.5: Sigmoidal curve fitting constants from Table Curve.

| | Dry Unit Weights (kN/m ³) | S/D Ratio | a | b | c | d | R ² |
|--|--|-----------|-------|------|------|-------|----------------|
| Area Replacement Index vs Normalized Average Pore Pressure | 12.56 | 1 | 0.17 | 0.92 | 0.48 | -0.19 | 1.00 |
| | | 1.18 | 0.14 | 0.93 | 0.61 | -0.22 | 0.99 |
| | | 1.45 | -1.37 | 2.55 | 1.36 | -0.48 | 0.99 |
| | | 1.62 | 0.53 | 0.52 | 0.63 | -0.20 | 0.99 |
| | | 2.17 | 0.54 | 0.48 | 0.91 | -0.23 | 0.99 |
| | 13.03 | 1 | 0.10 | 1.10 | 0.43 | -0.26 | 1.00 |
| | | 1.18 | 0.07 | 1.06 | 0.59 | -0.27 | 0.99 |
| | | 1.45 | 0.07 | 1.01 | 0.77 | -0.28 | 1.00 |
| | | 1.62 | 0.47 | 0.55 | 0.65 | -0.17 | 0.99 |
| | | 2.17 | 0.48 | 0.54 | 0.84 | -0.20 | 0.99 |
| | 13.35 | 1 | 0.14 | 1.04 | 0.40 | -0.23 | 1.00 |
| | | 1.18 | 0.17 | 0.90 | 0.53 | -0.20 | 1.00 |
| | | 1.45 | -0.12 | 1.28 | 0.82 | -0.37 | 0.99 |
| | | 1.62 | 0.49 | 0.55 | 0.59 | -0.18 | 0.99 |
| | | 2.17 | 0.44 | 0.58 | 0.87 | -0.21 | 0.99 |
| | 13.82 | 1 | 0.14 | 1.04 | 0.40 | -0.22 | 1.00 |
| | | 1.18 | 0.19 | 0.92 | 0.49 | -0.21 | 0.99 |
| | | 1.45 | 0.25 | 0.83 | 0.58 | -0.22 | 0.99 |
| | | 1.62 | 0.48 | 0.54 | 0.56 | -0.16 | 1.00 |
| | | 2.17 | 0.62 | 0.39 | 0.70 | -0.15 | 1.00 |
| Area Replacement Index vs. Normalized q/L | 12.56 | 1 | 0.15 | 0.94 | 0.44 | -0.17 | 1.00 |
| | | 1.18 | 0.14 | 0.96 | 0.54 | -0.21 | 0.99 |
| | | 1.45 | 0.00 | 1.13 | 0.73 | -0.30 | 0.99 |
| | | 1.62 | 0.48 | 0.57 | 0.61 | -0.20 | 0.99 |
| | | 2.17 | 0.55 | 0.47 | 0.82 | -0.23 | 0.99 |
| | 13.03 | 1 | 0.08 | 1.15 | 0.38 | -0.25 | 1.00 |
| | | 1.18 | 0.10 | 1.00 | 0.53 | -0.23 | 1.00 |
| | | 1.45 | 0.09 | 1.00 | 0.69 | -0.26 | 0.99 |
| | | 1.62 | 0.42 | 0.60 | 0.63 | -0.17 | 0.99 |
| | | 2.17 | 0.51 | 0.51 | 0.76 | -0.18 | 0.99 |
| | 13.35 | 1 | 0.13 | 1.10 | 0.33 | -0.21 | 0.99 |
| | | 1.18 | 0.17 | 0.93 | 0.47 | -0.18 | 1.00 |
| | | 1.45 | 0.11 | 1.01 | 0.61 | -0.28 | 1.00 |
| | | 1.62 | 0.37 | 0.70 | 0.57 | -0.23 | 1.00 |
| | | 2.17 | 0.51 | 0.51 | 0.75 | -0.18 | 0.99 |
| | 13.82 | 1 | 0.09 | 1.14 | 0.35 | -0.21 | 0.99 |
| | | 1.18 | 0.12 | 1.04 | 0.44 | -0.23 | 0.99 |
| | | 1.45 | 0.18 | 0.92 | 0.54 | -0.22 | 0.99 |
| | | 1.62 | 0.38 | 0.65 | 0.55 | -0.17 | 1.00 |
| | | 2.17 | 0.53 | 0.48 | 0.69 | -0.16 | 1.00 |

Figure 4.15 shows the relationship between normalized average pore pressure and normalized q/L. The normalized average pore pressure and normalized q/L values that

are were shown seem to be proportional. Normalized average pore pressure was plotted versus normalized q/L to determine whether or not the two parameters were indeed proportional. This proportionality seems to occur over all five S/D ratios and four unit weights.

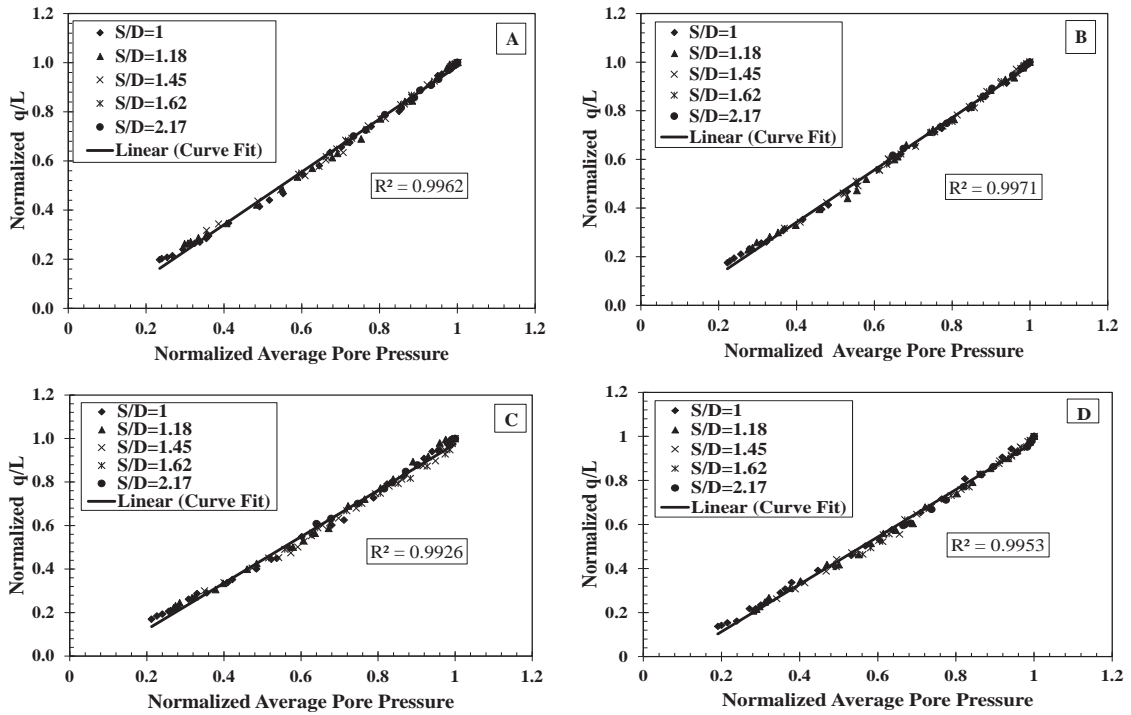


Figure 4.15: Linear relationship between normalized average pore pressure and normalized q/L grouped by S/D ratios for a dry unit weight of (A) 12.56 kN/m^3 , (B) 13.03 kN/m^3 , (C) 13.35 kN/m^3 and (D) 13.82 kN/m^3 .

Looking at Figure 4.15, the proportionality between normalized average pore pressure and normalized q/L seems to occur over all five S/D ratios and four dry unit weights. The R^2 values presented in Figure 4.15, suggest that the relationship between normalized pore pressure and normalized q/L is highly linear for all S/D ratios and dry unit weights. For all five S/D ratios at all four dry unit weights, R^2 values were greater than 0.99. As the

normalized pore pressure decreases, the normalized q/L decreases. According to Quinones-Rozo (2010) research, a linear relationship between pore pressure and q/L describes laminar flow. Laminar flow occurs when fluid is moving at low velocity. When fluids move at low velocities, no lateral mixing takes place. Also, Quinones-Rozo (2010) research found out that Lugeon values are approximately equal regardless of pore pressure during laminar flow. Figure 4.16 was developed to verify that the flow within the laboratory-scale physical seepage model was indeed laminar.

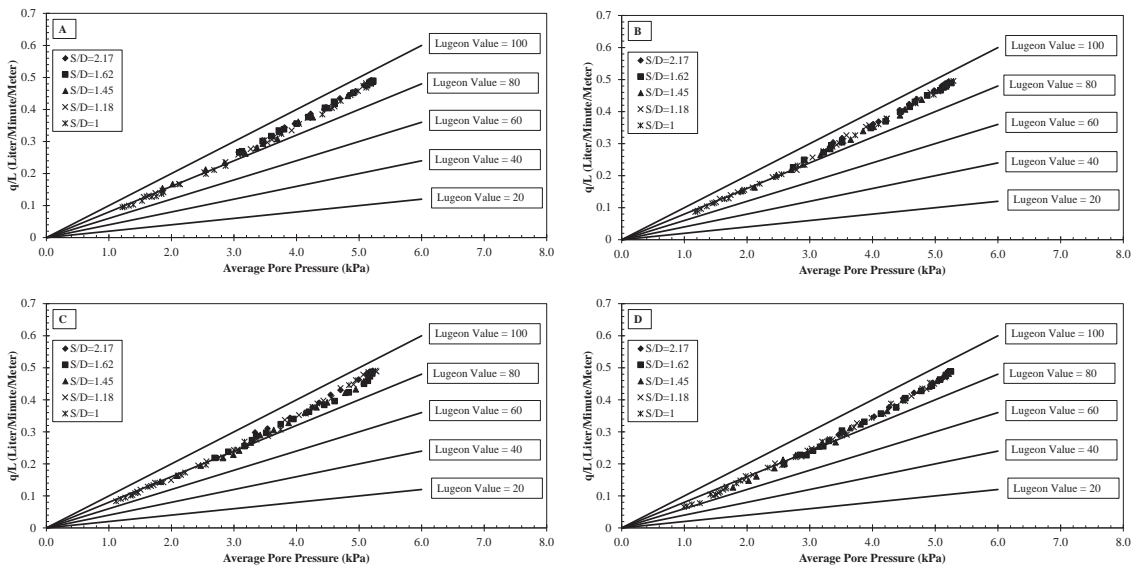


Figure 4.16: Relationship between average pore pressure and q/L grouped by S/D ratios for a dry unit weight of (A) 12.56 kN/m³, (B) 13.03 kN/m³, (C) 13.35 kN/m³ and (D) 13.82 kN/m³.

As seen in Figure 4.16, most data points across all S/D ratios and dry unit weights fell within the boundaries corresponding to a Lugeon value of 80 and 100. This figure illustrates that Lugeon values seen within the laboratory-scale physical seepage model were fairly constant at various average pore pressures. This behavior is consistent with the characterization of laminar flow described in Quinones-Rozo (2010) research.

Now that the flow behavior within the test box has been characterized, normalized pore pressure and normalized q/L were plotted versus normalized Lugeon values. As stated earlier in this thesis, Lugeon values are used in this study to help quantify the effectiveness of a grout curtain. Utilizing Lugeon values in this study allows implementation of the proposed efficiency parameters using current field techniques. Figure 4.17 illustrates the relationship between normalized average pore pressure versus normalized average Lugeon value grouped by S/D ratio at various dry unit weights.

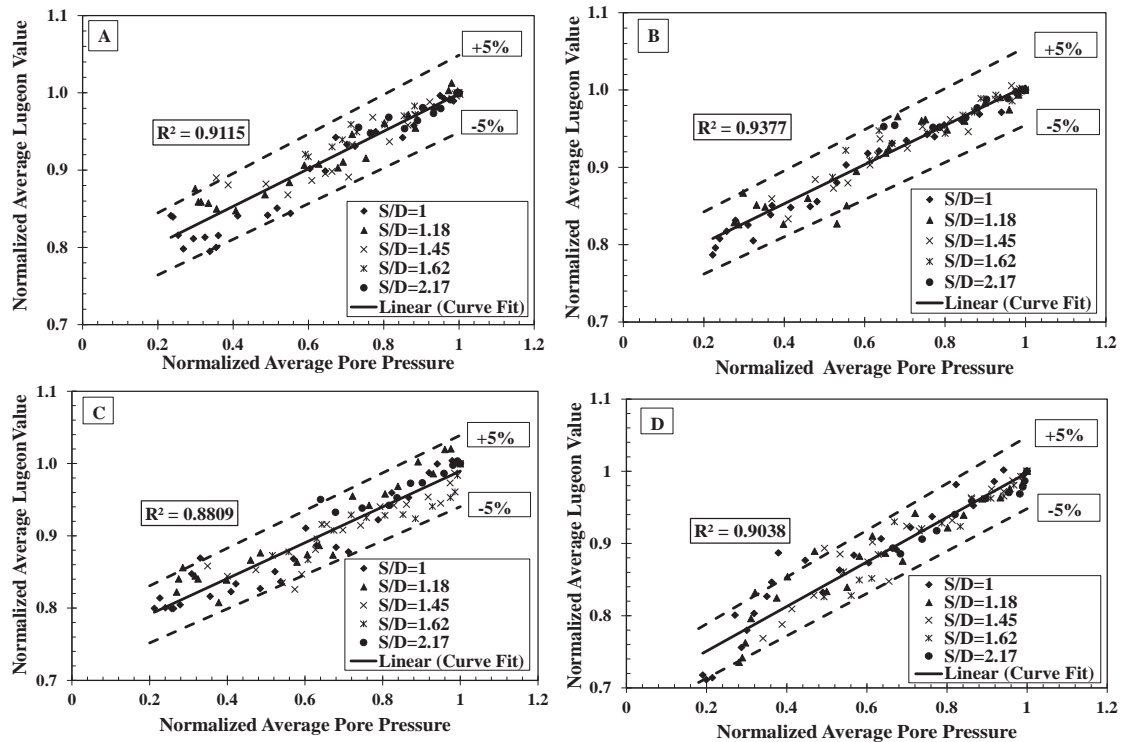


Figure 4.17: Relationship between normalized average pore pressure and normalized average Lugeon values grouped by S/D ratios for a dry unit weight of (A) 12.56 kN/m^3 , (B) 13.03 kN/m^3 , (C) 13.35 kN/m^3 and (D) 13.82 kN/m^3 .

As seen in Figure 4.17, it appears that there is a linear relationship between normalized average pore pressure and normalized average Lugeon value. However, the figure does

show some variability with respect to normalized Lugeon values. The normalized Lugeon values for all S/D ratios at various different dry unit weights do have some scatter. The scatter in these plots deal with the pressure range that was observed in the box. The maximum pore pressure that was observed in the box was 5.30 kPa. Looking back at the equation that was utilized to calculate Lugeon values for this study presented in Chapter 2, the equation uses a reference pressure of 1 MPa. Since, the test box is operating at such small pressures compared to the reference pressure; small changes in pore pressure can result in much larger changes in Lugeon values. Even though test data appears to scatter a little, a consistent trend has developed for all S/D ratios at all dry unit weights. As the normalized average pore pressure decreases, so does the normalized average Lugeon values. Linear curve fitting for the individual test data shown in Figure 4.17, produced high R^2 values. R^2 values for all S/D ratios at all dry unit weights were greater than 0.82 and can be seen in Table 4.6. These high R^2 values imply that a linear relationship between normalized average pore pressure and normalized average Lugeon value exists. Further analysis shows that at constant unit weight, as the S/D ratio increases, the slope of the linear best fit line decreases.

Table 4.6: Unit weight, S/D ratio, slope, Y-intercept and R^2 values for linear best fit lines of normalized average pore pressure versus normalized average Lugeon value plots.

| Dry Unit Weight (kN/m ³) | S/D Ratio | Slope | Y-Intercept | R^2 |
|--------------------------------------|-----------|-------|-------------|-------|
| 12.56 | 1 | 0.26 | 0.73 | 0.92 |
| | 1.18 | 0.22 | 0.78 | 0.92 |
| | 1.45 | 0.21 | 0.78 | 0.84 |
| | 1.62 | 0.19 | 0.81 | 0.96 |
| | 2.17 | 0.18 | 0.81 | 0.82 |
| 13.03 | 1 | 0.26 | 0.74 | 0.97 |
| | 1.18 | 0.24 | 0.76 | 0.87 |
| | 1.45 | 0.25 | 0.75 | 0.95 |
| | 1.62 | 0.21 | 0.79 | 0.92 |
| | 2.17 | 0.15 | 0.84 | 0.88 |
| 13.35 | 1 | 0.26 | 0.79 | 0.90 |
| | 1.18 | 0.25 | 0.74 | 0.90 |
| | 1.45 | 0.25 | 0.73 | 0.85 |
| | 1.62 | 0.21 | 0.76 | 0.84 |
| | 2.17 | 0.19 | 0.80 | 0.83 |
| 13.82 | 1 | 0.34 | 0.68 | 0.89 |
| | 1.18 | 0.33 | 0.67 | 0.93 |
| | 1.45 | 0.31 | 0.69 | 0.91 |
| | 1.62 | 0.30 | 0.69 | 0.96 |
| | 2.17 | 0.29 | 0.70 | 0.89 |

A similar relationship was developed between normalized q/L and normalized average Lugeon value. Looking back at an earlier relationship developed between normalized average pore pressures versus normalized q/L seen in Figure 4.15, it appears that the relationship is highly linear. Knowing this relationship, a plot comparing normalized q/L versus normalized average Lugeon value should produce a plot that has a similar relationship to the normalized average pore pressure versus normalized average Lugeon value plot seen in Figure 4.17. Figure 4.18 shows the relationship between

normalized q/L versus normalized average Lugeon value grouped by S/D ratio at various dry unit weights.

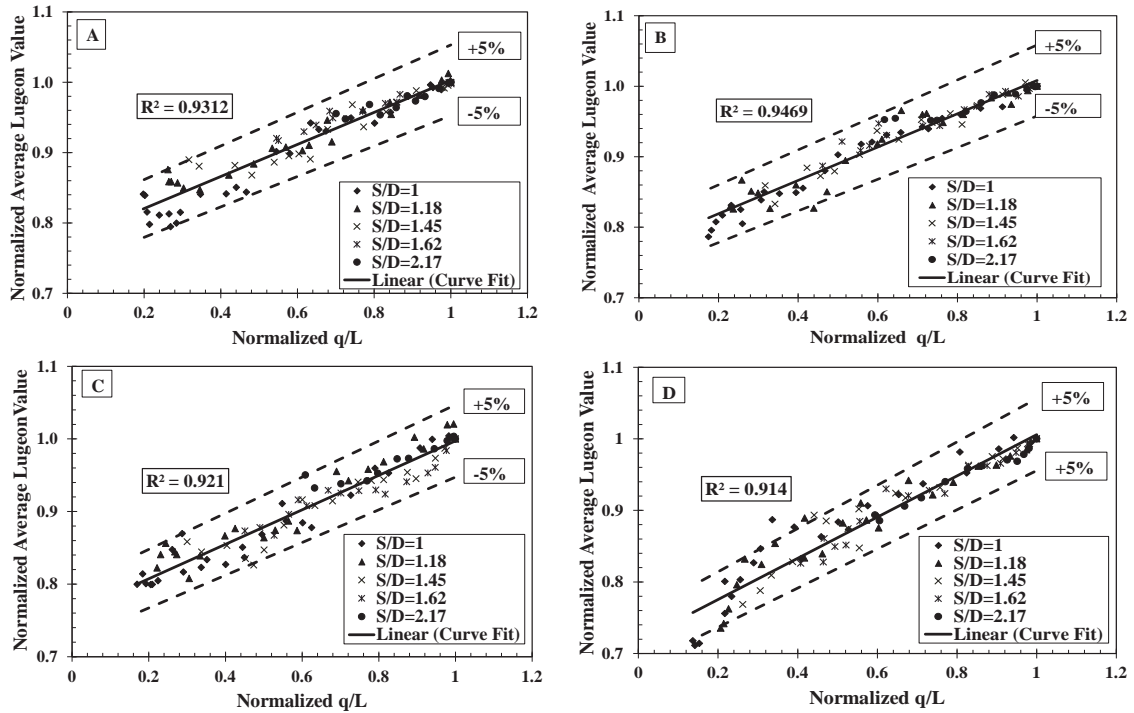


Figure 4.18: Relationship between normalized q/L and normalized average Lugeon values grouped by S/D ratios for a dry unit weight of (A) 12.56 kN/m^3 , (B) 13.03 kN/m^3 , (C) 13.35 kN/m^3 and (D) 13.82 kN/m^3 .

As seen in Figure 4.18, a linear relationship between normalized q/L and normalized average Lugeon value developed. The behavior of this relationship was quite similar to the relationship illustrated in Figure 4.17, as expected. As normalized q/L decreases, the normalized average Lugeon value decreases. This trend was consistent across all S/D ratios and dry unit weights. Linear curve fitting produced best fit lines with high R^2 values which can be seen in Table 3.10. All R^2 values were greater than 0.87.

Table 4.7: Unit weight, S/D ratio, slope, Y-intercept and R² values for linear best fit lines of normalized q/L versus normalized average Lugeon value plots.

| Dry Unit Weight (kN/m ³) | S/D Ratio | Slope | Y-Intercept | R ² |
|--------------------------------------|-----------|-------|-------------|----------------|
| 12.56 | 1 | 0.25 | 0.75 | 0.94 |
| | 1.18 | 0.21 | 0.79 | 0.95 |
| | 1.45 | 0.20 | 0.79 | 0.89 |
| | 1.62 | 0.17 | 0.83 | 0.97 |
| | 2.17 | 0.17 | 0.83 | 0.87 |
| 13.03 | 1 | 0.25 | 0.76 | 0.97 |
| | 1.18 | 0.23 | 0.77 | 0.91 |
| | 1.45 | 0.23 | 0.78 | 0.96 |
| | 1.62 | 0.19 | 0.81 | 0.94 |
| | 2.17 | 0.14 | 0.86 | 0.91 |
| 13.35 | 1 | 0.25 | 0.75 | 0.94 |
| | 1.18 | 0.25 | 0.76 | 0.94 |
| | 1.45 | 0.23 | 0.75 | 0.90 |
| | 1.62 | 0.19 | 0.78 | 0.89 |
| | 2.17 | 0.18 | 0.82 | 0.88 |
| 13.82 | 1 | 0.32 | 0.70 | 0.89 |
| | 1.18 | 0.28 | 0.72 | 0.90 |
| | 1.45 | 0.28 | 0.72 | 0.93 |
| | 1.62 | 0.28 | 0.72 | 0.95 |
| | 2.17 | 0.25 | 0.74 | 0.97 |

Plotting normalized average pore pressure and normalized q/L versus area replacement index and normalized average Lugeon value produced two distinct shapes. Normalized average pore pressure and normalized q/L plotted against area replacement index established non-linear sigmoidal shaped curves. While normalized average pore pressure and normalized q/L plotted against normalized average Lugeon value produced linear relationships.

Trends and relationships between normalized average pore pressure, normalized q/L , area replacement index and normalized average Lugeon values have been observed. To aid in the quantification in the effectiveness of a grout curtain, area replacement index was plotted versus normalized average Lugeon value for all S/D ratios at all dry unit weights. Figure 4.19 illustrates this relationship. Linear best fit lines forced through a normalized average Lugeon value of one are also seen in Figure 4.19. Equations for the linear best fit lines along with their corresponding R^2 values are tabulated in Table 4.8. Linear best fit lines were forced through a Lugeon value of one to allow for proper scaling and comparison to various case history data which will be discussed later.

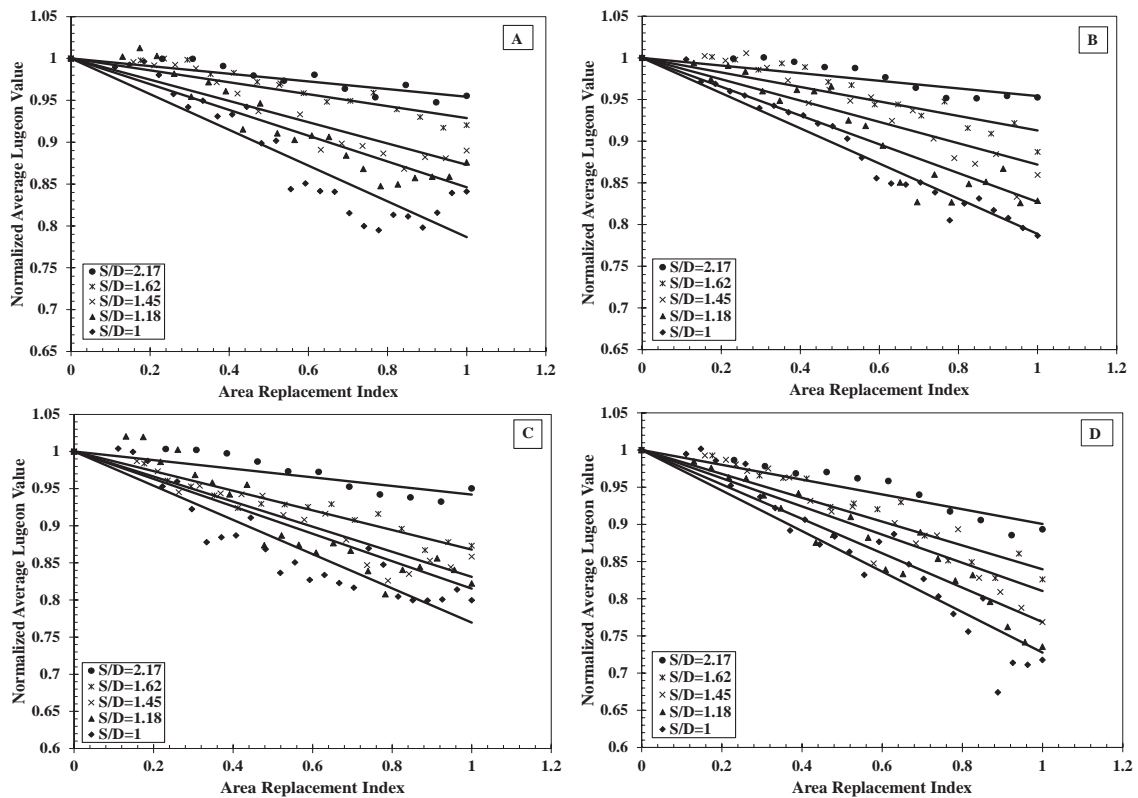


Figure 4.19: Relationship between area replacement index and normalized average Lugeon values grouped by S/D ratios for a dry unit weight of (A) 12.56 kN/m^3 , (B) 13.03 kN/m^3 , (C) 13.35 kN/m^3 and (D) 13.82 kN/m^3 .

Table 4.8: Unit weight, S/D ratio, slope, Y-intercept and R^2 values for linear best fit lines of area replacement index versus normalized average Lugeon value plots.

| Dry Unit Weight (kN/m ³) | S/D Ratio | Slope | Y-Intercept | R^2 |
|--------------------------------------|-----------|-------|-------------|-------|
| 12.56 | 1 | -0.21 | 1.00 | 0.86 |
| | 1.18 | -0.15 | 1.00 | 0.87 |
| | 1.45 | -0.13 | 1.00 | 0.85 |
| | 1.62 | -0.07 | 1.00 | 0.86 |
| | 2.17 | -0.05 | 1.00 | 0.81 |
| 13.03 | 1 | -0.21 | 1.00 | 0.96 |
| | 1.18 | -0.17 | 1.00 | 0.84 |
| | 1.45 | -0.13 | 1.00 | 0.82 |
| | 1.62 | -0.09 | 1.00 | 0.82 |
| | 2.17 | -0.05 | 1.00 | 0.78 |
| 13.35 | 1 | -0.23 | 1.00 | 0.86 |
| | 1.18 | -0.18 | 1.00 | 0.83 |
| | 1.45 | -0.17 | 1.00 | 0.90 |
| | 1.62 | -0.13 | 1.00 | 0.92 |
| | 2.17 | -0.06 | 1.00 | 0.75 |
| 13.82 | 1 | -0.27 | 1.00 | 0.90 |
| | 1.18 | -0.23 | 1.00 | 0.91 |
| | 1.45 | -0.19 | 1.00 | 0.86 |
| | 1.62 | -0.16 | 1.00 | 0.89 |
| | 2.17 | -0.10 | 1.00 | 0.88 |

R^2 values for all linear best fit lines were greater than 0.81, with the exception of two tests, S/D = 2.17 at a dry unit weight of 13.03 kN/m³ and S/D = 2.17 at a dry unit weight of 13.35 kN/m³. High R^2 values imply that a linear correlation between area replacement index and normalized average pore pressure as a function of S/D ratio exists. At this point, at a given S/D ratio, the relationship between area replacement index and normalized average Lugeon value can be described as shown in Equation 17.

$$LV_n = m_1(ARI)+1 \quad (17)$$

where LV_n = normalized average Lugeon value, m_1 = slope of linear best fit line from the area replacement index versus normalized average Lugeon value plot and ARI = area replacement index.

Looking at the slopes of the linear best fit lines in Figure 4.19, at a constant S/D ratio, it appears that the slopes increase, as dry unit weight increases. Upon speculation, it is believed that as the dry unit weight increased, the void space between soil particles decreased resulting in less volume discharge, hence a decreased Lugeon value. This relationship is evident for all S/D ratios. Plotting S/D ratio versus m_1 as a function of dry unit weight shows another linear correlation seen in Figure 4.20.

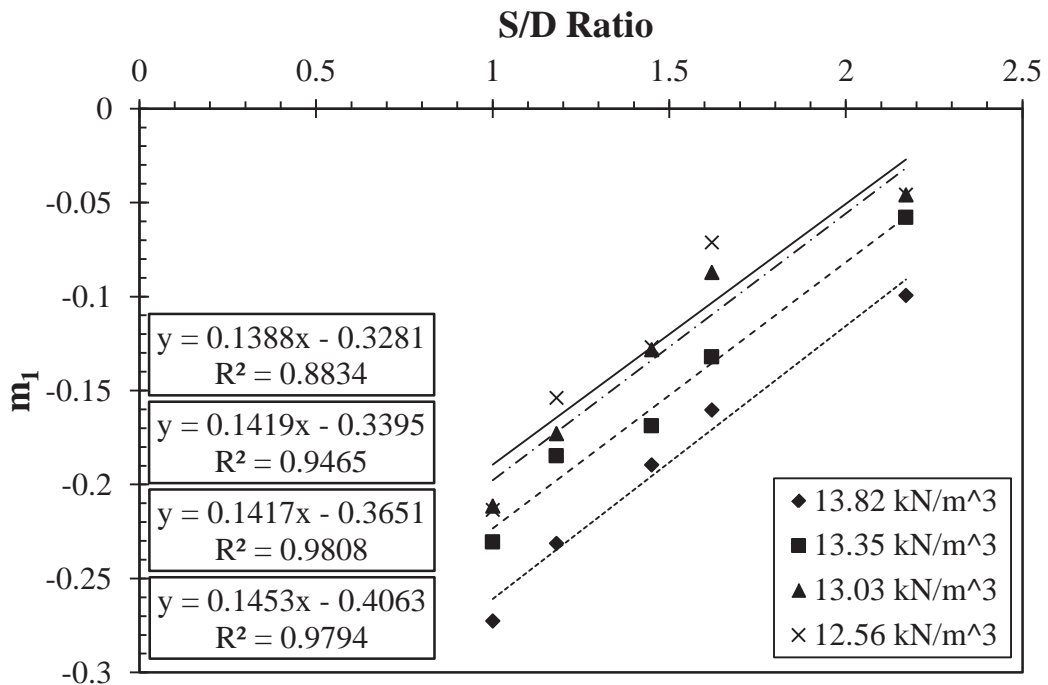


Figure 4.20: S/D ratio versus m_1 , grouped by dry unit weight.

As seen in Figure 4.20, R_2 values for all 4 dry unit weights showed high linear correlation. Also, the slopes of the S/D ratio versus m_1 as a function of dry unit weight

were approximately equal. Since the slopes of the best fit lines were approximately equal, an average slope value was obtained. The average slope obtained from the S/D ratio versus m_1 plot will be referred to as m_{2avg} which is equal to 0.1419. Equation 18 shows the linear relationship between S/D ratio versus m_1 as a function of dry unit weight.

$$m_1 = m_{2avg} \left(\frac{S}{D} \right) + b_2 \quad (18)$$

where S/D = spacing-to-diameter ratio and b_2 = y-intercept as a function of dry density.

Now, substitute Equation 18 into Equation 17 to get Equation 19 which takes into account S/D ratio and area replacement index.

$$LV_n = \left(m_{2avg} \left(\frac{S}{D} \right) + b_2 \right) ARI + 1 \quad (19)$$

The next step to this analysis was plotting dry unit weight versus b_2 . This relationship can be seen in Figure 4.21.

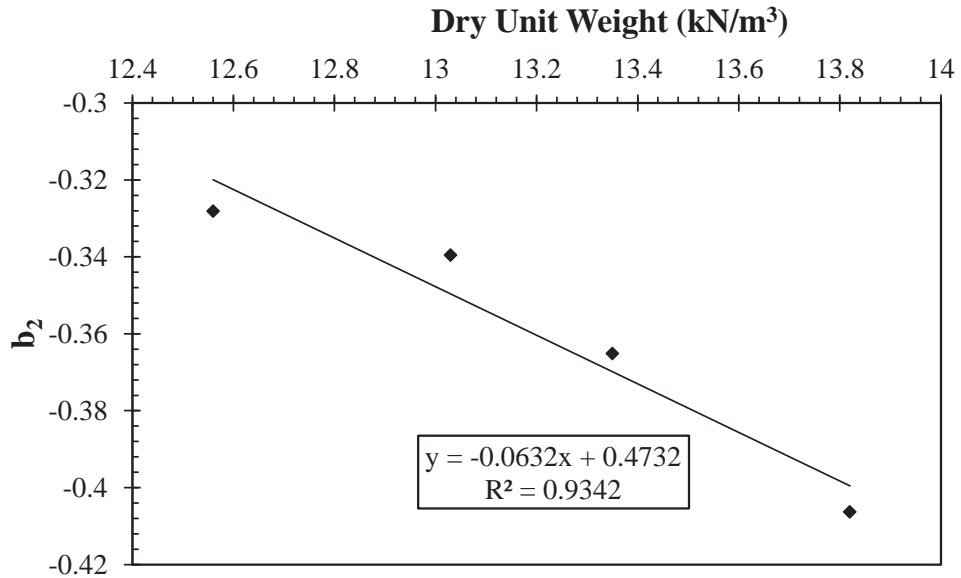


Figure 4.21: Dry unit weight versus b_2 .

Once again, a highly linear correlation between dry unit weight and b_2 resulted. The equation for the best fit line through the test data is shown in Figure 4.21, which displays a relationship where dry unit weight increases as b_2 decreases. The slope and y-intercept presented in Figure 4.21 were assumed to be material constants of the test media used in this research and will be referred to as m_3 and b_3 , respectively. For this research $m_3 = -0.0632$ and $b_3 = 0.4732$. However, further research should be focused on determining m_3 and b_3 material constants for various materials. Equation 20 shows the linear relationship between dry unit weight and b_2 .

$$b_2 = m_3(\gamma_{dry}) + b_3 \quad (20)$$

where m_3 = material constant, b_3 = material constant and γ_{dry} =dry unit weight.

At this point substitute Equation 20 into Equation 19 to get Equation 21 which gives the final relationship that calculates a normalized average Lugeon value.

$$LV_n = \left(m_{2avg} \left(\frac{S}{D} \right) + m_3(\gamma_{dry}) + b_3 \right) ARI + 1 \quad (21)$$

where LV_n = normalized average Lugeon value, ARI = area replacement index, m_3 = material constant = -0.0632, b_3 = material constant = 0.4732, $m_{2avg} = 0.1419$, S/D = spacing-to-diameter ratio and γ_{dry} = dry unit weight.

Equation 21 is a function of S/D ratio, dry unit weight and area replacement index. This relationship quantifies the effectiveness of a grout curtain as a function of three parameters which include S/D ratio, dry unit weight and area replacement index.

4.7 Summary

It is possible to quantifiably predict the effectiveness of a grout curtain using a physical model knowing a few properties of the underlying soil material and grout curtain. Several trends and relationships developed when comparing normalized average pore pressure, normalized q/L , area replacement index and normalized average Lugeon values to one another at various S/D ratios and dry unit weights. These relationships along with further analysis, assisted in the creation of a new mathematical equation that helps predict the effectiveness of a grout curtain in a physical model. However, this new relationship has yet to be proven accurate using data from real life structures. To show that this relationship can be scaled up to real life structures, case history data needed to be obtained to assess the validity of this proposed relationship.

Chapter 5: Relationship Verification

To verify the accuracy of the newly developed relationship, case history data was collected and analyzed. The use of actual field data from different case history studies will further validate the relationship. Using case history data presented in Chapter 2 will help demonstrate that it is possible to predict the effectiveness of a grout curtain using additional data that was not used in the creation of this quantifiable relationship.

All four case history studies that were analyzed to assess the applicability of the proposed relationship (Equation 21) are dam structures located in South Korea. The specific names of the dams were not identified in the Hong et al. (2003) research, but rather identified as Site 1, Site 2, Site 3 and Site 4. Site 1, Site 3 and Site 4 are all concrete-faced rock fill dams while Site 2 is a gravity concrete dam. Metamorphic and sedimentary rock formations underlie all four dam structures. Table 2.1 presented in Chapter 2 provided pertinent information about the case history dams. This table is presented again here as Table 5.1 for continuity.

Table 5.1: Site conditions and grouting properties of four case history studies (Hong et al. 2003).

| | Site 1 | Site 2 | Site 3 | Site 4 |
|--------------------------------------|----------------------------------|--|----------------------------------|--|
| Type of Dam | Concrete Face Rock Fill Dam | Gravity Concrete Dam | Concrete Face Rock Fill Dam | Concrete Face Rock Fill Dam |
| Type of Rock | Metamorphic | Sedimentary | Sedimentary | Sedimentary |
| Number of Holes | 231 | 35 | 28 | 41 |
| Depth of Holes (m) | 40 | 20 | 20 | 40 |
| Grout Spacing | 1 column with 1.5 m spaced holes | 2 columns at 3.0 m zigzag interval; 3.0 m spaced holes | 1 column with 1.5 m spaced holes | 2 columns at 3.0 m zigzag interval; 3.0 m spaced holes |
| Span of Grout Curtain (m) | 347 | 105 | 42 | 123 |
| Average Lugeon Value Before Grouting | 3 | 3.9 | 11 | 2.37 |
| Average Lugeon Value After Grouting | 1 | 1.9 | 3 | 1 |
| Injection Pressure (Mpa) | 0.39-2.45 | 0.15-0.59 | 0.15-0.59 | 0.29-2.45 |
| Assumed Grout Column Diameter (m) | 1.016 | 1.016 | 1.016 | 1.016 |

Four separate parameters for each site had to be identified, which included normalized average Lugeon value, S/D ratio, area replacement index and dry unit weight. S/D ratios and area replacement index values were calculated for all four sites based on the geometry of the individual grout curtains which was presented in Chapter 2. The diameters of the grout columns include allowances for horizontal spread of the grout material. Horizontal spread is the maximum distance the grout material can penetrate the soil media. Typical grout curtain designs account for 1.016 m of horizontal spread (Hacker, 2014). Thus, the diameters of the grout columns for the case histories were assumed to be 1.016 m. Normalized average Lugeon values were calculated by dividing the average Lugeon value after grouting by the average Lugeon value before grouting. Dry unit weights of the underlying material were not presented. However, underlying rock types were given. With this knowledge, average dry unit weights for these underlying rock formations were assumed based on research done by Magner (1963) and

Akeinyemi et al. (2012). Metamorphic and sedimentary rocks were assumed to have unit weights of 23.56 kN/m^3 and 22.77 kN/m^3 , respectively. These unit weights are well outside the unit weights tested in this research. To accommodate the assumed unit weights of the metamorphic and sedimentary rock formations, predicted curves were developed holding the S/D ratio and ARI constant in Equation 21. Predicted curves matching the unit weights of the underlying rock material allowed for easy comparison.

Figure 5.1 illustrates the case history data plotted against predicted values.

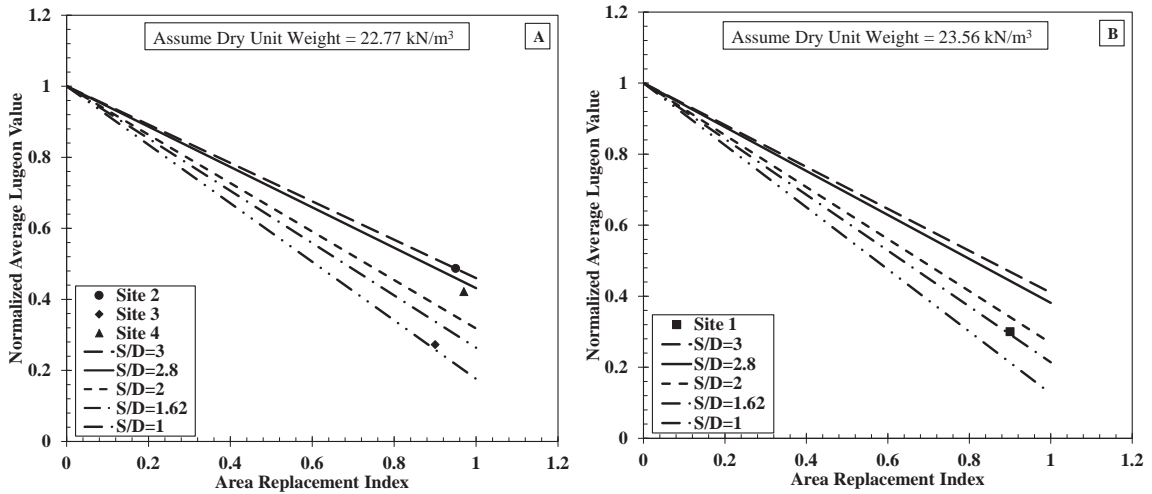


Figure 5.1: Case history data versus predicted values plotted at dry unit weights of (A) 22.77 kN/m^3 and (B) 23.56 kN/m^3 .

As seen in Figure 5.1, all case history studies have S/D ratios between 1 and 3. Since the underlying rock material seen at Site 2, Site 3 and Site 4 were similar; these sites were plotted together, seen in Figure 5.1A. Site 1 composed of metamorphic rock, was plotted by itself seen in Figure 5.1B. To truly assess the applicability of the proposed relationship, predicted normalized average Lugeon values were plotted versus actual normalized average Lugeon values for all four case histories. Predicted values are compared to actual values in Figure 5.2.

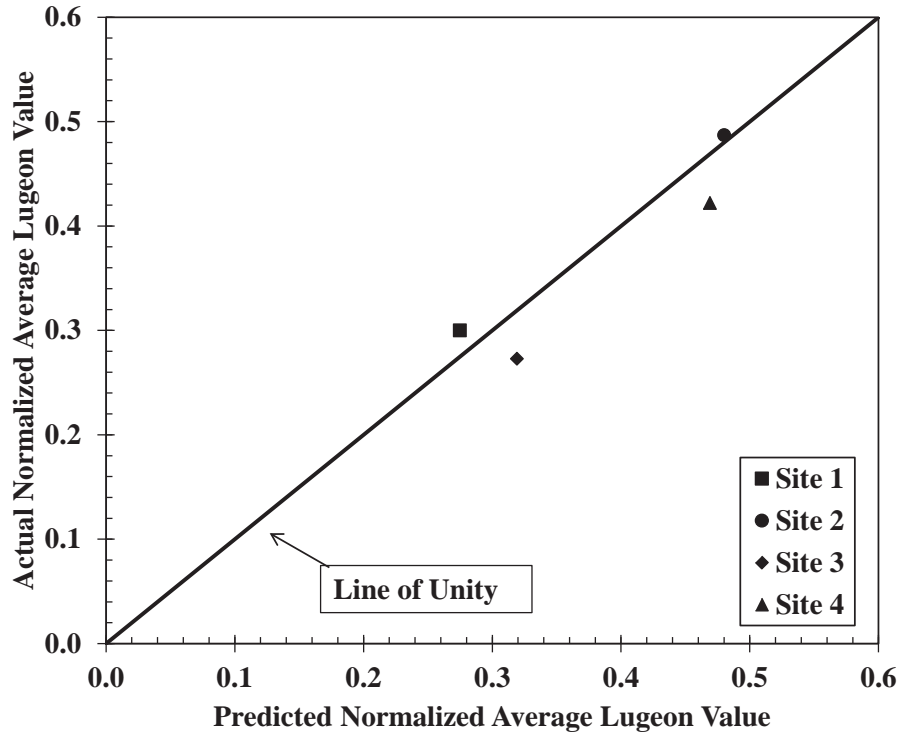


Figure 5.2: Relationship between predicted normalized average Lugeon value versus actual normalized average Lugeon values.

Figure 5.2 illustrates the applicability of the relationship presented in Equation 21 using data from four separate case histories. As is seen in the figure, all four points plot near the line of unity. The absolute percent error was calculated for all four case histories using Equation 22.

$$Absolute \% Error = \left| \frac{Actual Value - Expected Value}{Actual Value} \right| \times 100 \quad (22)$$

Table 5.2, tabulates absolute percent error for each of the 4 case histories.

Table 5.2: Calculated absolute percent errors for all case history studies.

| | Predicted Normalized average Lugeon value | Actual Normalized average Lugeon value | Absolute percent error (%) |
|--------|---|--|----------------------------|
| Site 1 | 0.27 | 0.30 | 8.40 |
| Site 2 | 0.48 | 0.49 | 1.42 |
| Site 3 | 0.32 | 0.27 | 17.07 |
| Site 4 | 0.47 | 0.42 | 11.20 |

Evaluating the absolute percent error values tabulated in Table 5.2, it is evident that the relationship presented in Equation 21 is reliable when predicting normalized average Lugeon values. Site 3 shows the highest absolute percent error of 17.1 percent. Site 2 on the other hand shows the lowest absolute percent error of 1.42 percent. Site 1 and Site 4 have absolute percent error values that fall in between the absolute percent errors of Site 2 and Site 3.

Normalized average Lugeon values can be predicted as a function of three parameters, S/D ratio, dry unit weight and area replacement index to assess the effectiveness of a grout curtain. Case history data further validates that the relationship that developed while testing in a box, can be scaled up to real life structures. Being able to reliably predict Lugeon values will prove very beneficial when designing grout curtain structures in the future.

Chapter 6: Summary and Conclusions

This thesis presented the results of a laboratory study that quantified the effectiveness of a grout curtain using a laboratory-scale physical seepage model. This study investigated changes in pore pressures and discharge volumes to determine corresponding Lugeon values at various unit weights and spacings. The development of a new performance parameter similar to the area replacement ratio used in ground improvement methods was utilized to help develop a quantifiable relationship. This research discovered a linear relationship that takes into account soil unit weight, grout curtain spacing and a new performance parameter that can be used to help predict the effectiveness of a grout curtain. Verification of this relationship was done by using various case history studies.

The information presented in Chapter 2, discussed the current methodologies in which grout curtains are assessed. However, it is pointed out that most of the prior research was predominately based on qualitative means such as prior experience and rules of thumb rather than quantitative means. Quantifying the effectiveness of a grout curtain prior to installation would prove very beneficial. It would help engineers minimize factors such as time, cost and labor.

Chapter 3 presented the development of a physical model. This physical model was used to help determine and illustrate various geotechnical trends that exist within the test media with various grout curtain configurations. Measurements of pore pressure and discharge volume for various S/D ratios at different dry unit weights were taken. These measurements were subsequently used to calculate Lugeon values, which were the primary basis for quantification.

It was presented in Chapter 2, that an area replacement parameter similar to Priebe (1991) and Priebe (1995) area replacement ratio for ground improvement, could be used as a means to quantify the effectiveness of a grout curtain. Therefore, a new performance parameter known as the area replacement index was developed and presented in Chapter 4.

The main goal of this research was to generate a new quantifiable relationship to predict the effectiveness of a grout curtain based on Lugeon values and area replacement index values. This research illustrated a linear relationship that is a function of three parameters S/D ratio, dry unit weight and area replacement index. Knowing these three parameters allows for the prediction of the effectiveness of a grout curtain.

Case history data was obtained and analyzed for four various sites to assess the accuracy of the newly developed quantifiable relationship. Plotting actual data from four various case histories versus predicted data utilizing the newly developed quantifiable relationship exemplified a fairly accurate relationship. This comparison provided the necessary evidence to show the relationship can be scaled.

Thus, it is possible to accurately predict the effectiveness of a grout curtain knowing three parameters S/D ratio, dry unit weight and area replacement index. Quantifying the effectiveness of a grout curtain gives engineers the ability to optimize their design, while providing an adequate solution.

The research that was presented in this thesis only covered five S/D ratios at five dry unit weights for one test media. This does not cover a wide enough spectrum to consider this a relationship to be used by industry for design. However, there is evidence presented in this thesis that supports the use of this newly developed quantifiable

relationship to assess the effectiveness of a grout curtain. This is a significant step forward in the direction of assessing the effectiveness of a grout curtain quantifiably rather than qualitatively. The time and money that would be saved, further justify more research to be completed for this quantifiable relationship.

Future research should be aimed at further understanding the relationship between normalized Lugeon values with respects to S/D ratio and dry unit weight. This research showed that normalized average Lugeon values vary linearly as a function of S/D ratio, dry unit weight and area replacement index. However, the ranges of S/D ratios and dry unit weights were limited. Having a wider range of tests will allow for a better understanding of this relationship. Along the same lines, more test medias should be tested following the same procedures outlined in this research. As mentioned earlier, the quantifiable relationship that predicts the effectiveness of a grout curtain takes into account two material constants b_3 and m_3 . Using different test medias will help determine these material constants and will provide further understanding of this quantifiable relationship. This proposed research could ultimately lead to a more accurate and useful relationship.

Appendix A:

Index Testing on Test Sand

Table A.1: Particle grain size analysis data (ASTM D6913) for Kentucky River sand.

| Sieve Number | Opening (mm) | Weight Retained Each Sieve (g) | Weight of Soil Retained (g) | Weight of Soil Passed (g) | Percent Finer |
|--------------|--------------|--------------------------------|-----------------------------|---------------------------|---------------|
| 3/8" | 9.500 | 0.00 | 0 | 1471 | 100.000 |
| No. 4 | 4.750 | 0.00 | 0 | 1471 | 100.000 |
| No. 10 | 2.000 | 0.00 | 0 | 1471 | 100.000 |
| No. 20 | 0.850 | 11.80 | 11.8 | 1459.2 | 99.198 |
| No. 40 | 0.425 | 18.10 | 29.9 | 1441.1 | 97.967 |
| No. 60 | 0.250 | 183.20 | 213.1 | 1257.9 | 85.513 |
| No. 100 | 0.150 | 997.10 | 1210.2 | 260.8 | 17.729 |
| No. 140 | 0.106 | 127.60 | 1337.8 | 133.2 | 9.055 |
| No. 200 | 0.075 | 66.30 | 1404.1 | 66.9 | 4.548 |
| Pan | 0.000 | 66.90 | 1471 | 0 | 0.000 |
| | Total | 1471 | | | |

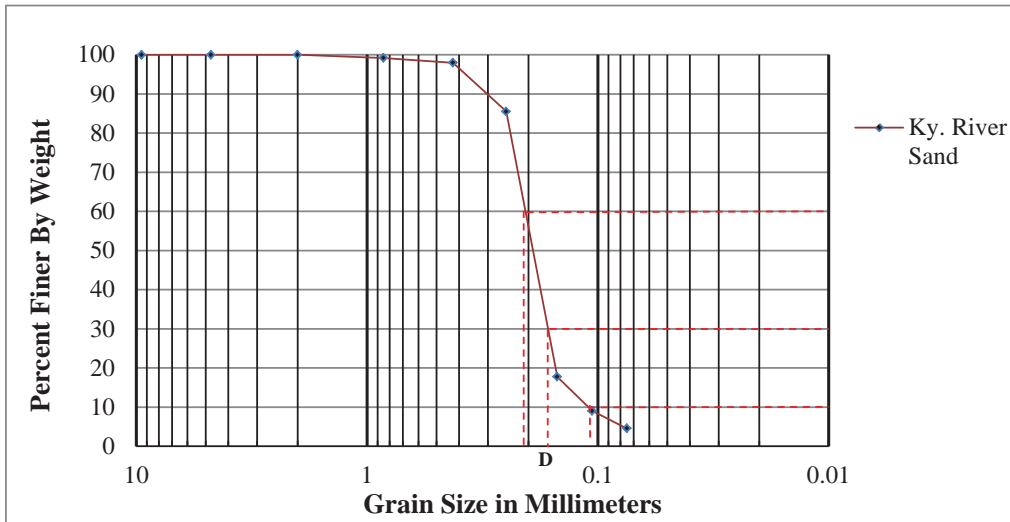


Figure A.1: Graphical representation of the particle grain size analysis test for Kentucky River sand.

Table A.2: Specific gravity data (ASTM D854) for Kentucky River sand.

| Specific Gravity | | | |
|---------------------------------|----------------|--------------|----------------|
| Soil Description | Ky. River Sand | | Ky. River Sand |
| Test number | 1 | | 3 |
| Nominal Pycometer Volume | 500 ml | 500 ml | 500 ml |
| Oven Dry Weight of Soil | 100 g | 96.3 g | 89.2 g |
| Weight of Pycometer+ Water | 665 g | 660.7 g | 662.6 g |
| Weight of Pycometer+ Water+Soil | 727.4 g | 721.1 g | 718.4 g |
| Temperature | 20 deg. Cels | 20 deg. Cels | 20 deg. Cels |
| Correction Factor K | 1 | 1 | 1 |
| Specific Gravity | 2.66 | | 2.67 |

Table A.3: Particle grain size analysis data (ASTM D6913) for Ohio River Valley Sand.

| Sieve Number | Opening (mm) | Weight Retained Each Sieve (g) | Weight of Soil Retained (g) | Weight of Soil Passed (g) | Percent Finer |
|--------------|--------------|--------------------------------|-----------------------------|---------------------------|---------------|
| 3/8" | 9.500 | 0.00 | 0 | 1073.58 | 100.000 |
| No. 4 | 4.750 | 38.92 | 38.92 | 1034.66 | 96.375 |
| No. 10 | 2.000 | 139.27 | 178.19 | 895.39 | 83.402 |
| No. 20 | 0.850 | 228.65 | 406.84 | 666.74 | 62.104 |
| No. 40 | 0.425 | 451.43 | 858.27 | 215.31 | 20.055 |
| No. 60 | 0.250 | 179.37 | 1037.64 | 35.94 | 3.348 |
| No. 100 | 0.150 | 32.46 | 1070.1 | 3.48 | 0.324 |
| No. 140 | 0.106 | 1.35 | 1071.45 | 2.13 | 0.198 |
| No. 200 | 0.075 | 0.33 | 1071.78 | 1.8 | 0.168 |
| Pan | 0.000 | 1.80 | 1073.58 | 0 | 0.000 |
| | Total | 1073.58 | | | |

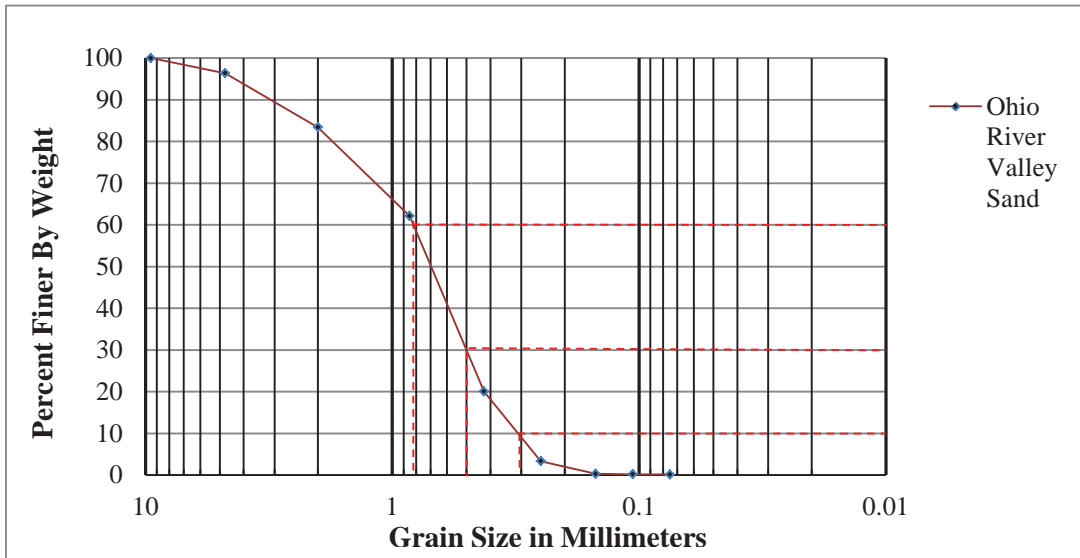


Figure A.2: Graphical representation of the particle grain size analysis test for Ohio River Valley sand.

Table A.4: Specific gravity data (ASTM D854) for Ohio River Valley sand.

| Specific Gravity | | | |
|---------------------------------|------------------------|-----------|------------------------|
| Soil Description | Ohio River Valley Sand | | Ohio River Valley Sand |
| Test number | 1 | | 2 |
| Nominal Pycometer Volume | 500 | ml | 500 |
| Oven Dry Weight of Soil | 103.2 | g | 112.3 |
| Weight of Pycometer+ Water | 678 | g | 683 |
| Weight of Pycometer+ Water+Soil | 742.3 | g | 753.1 |
| Temperature | 20 | deg. Cels | 20 |
| Correction Factor K | 1 | | 1 |
| Specific Gravity | 2.65 | | 2.66 |
| | | | 2.63 |

Appendix B:

Step-by-step Manual for Program Development in LabVIEW 2012

The pore pressure transducers used in this research were the Kulite XCL-11-250-150SG sealed gauge pore pressure transducers rated at 150 pounds per square inch. The three pore pressure transducers used in this research all contained calibration certificates provided by the manufacturer Kulite. The various information Kulite provided on these certificates will be essential data for the verification of Kulites calibration.

Wiring Modules

The first step in the calibration process of the Kulite XCL-11-250-150SG sealed gauge pore pressure transducers is to correctly assemble the module in the chaise from National Instruments(NI). The module that was used in the proper calibration of the Kulite XCL-11-250-150SG sealed gauge pore pressure transducers was the NI 9237 module. The NI 9237 module is a 4-channel, 24-Bit Half/Full-Bridge Analog Input Module. Figure B.1 shows the proper location within the NI chaise for the NI 9237 module.



Figure B.1: Proper Location for the NI 9237 Module in the NI chaise

The second step is to properly wire the three Kulite XCL-11-250-150SG sealed gauge pore pressure transducers to the sub-connector that will eventually be inserted into the NI 9237 module. The three Kulite pore pressure transducers were wired in a fully active four arm wheatstone bridge. Each pore pressure transducer has five individual

wires running from the pore pressure head with their own distinct color. Figure B.2 is showing the five different wires running from the Kulite XCL-11-250-150SG sealed gauge pore pressure transducer.

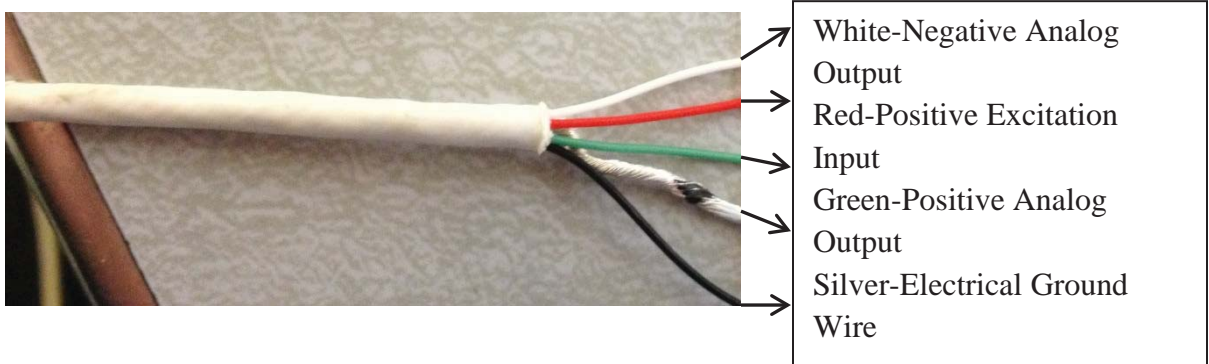


Figure B.2: Wires within each Kulite XCL-11-250-150SG sealed gauge pore pressure transducer

The red wire corresponds to the positive excitation input, the black wire corresponds to the negative excitation input, the green wire corresponds to the positive analog output, the white wire corresponds to the negative analog output, and the thick silver wire is the electrical ground wire.

Once one is familiar with the different types of wires that are within each Kulite XCL-11-250-150SG sealed gauge pore pressure transducer, it is time to wire all three pore pressure transducers to the NI sub-connector. Figure B.3 shows the proper way to wire each of the three Kulite XCL-11-250-150SG sealed gauge pore pressure transducers in a fully active four arm wheatstone bridge configuration.

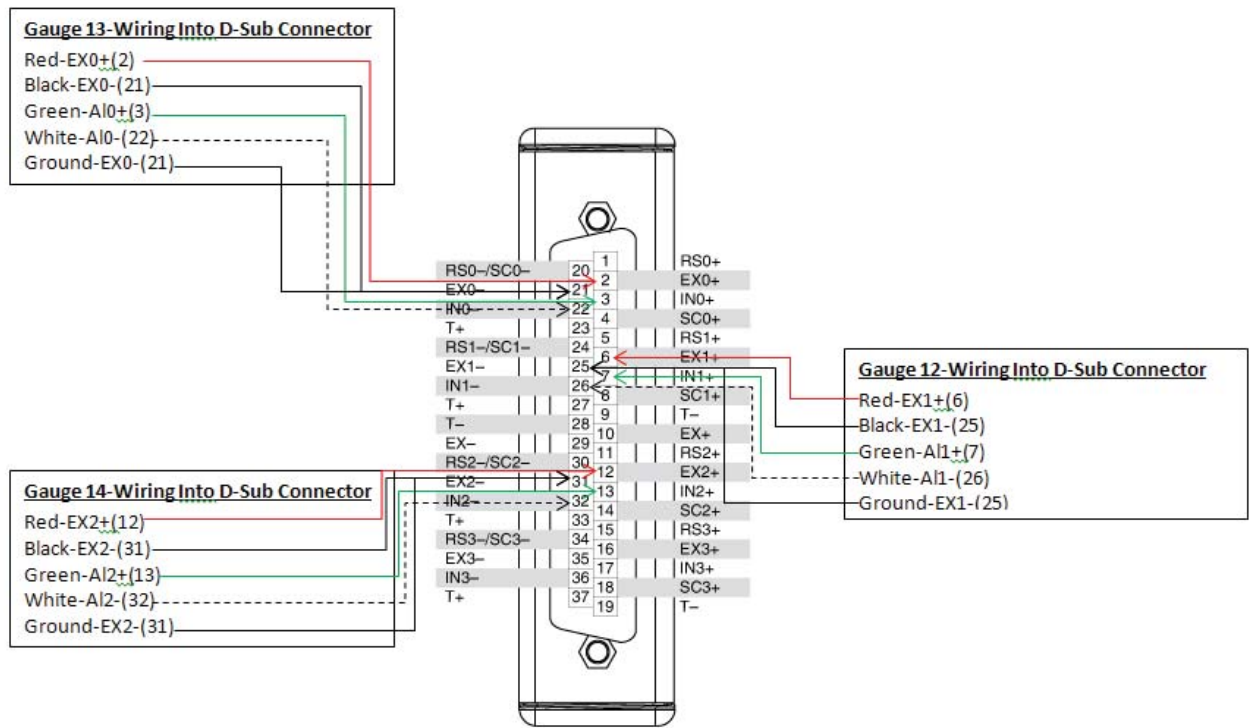
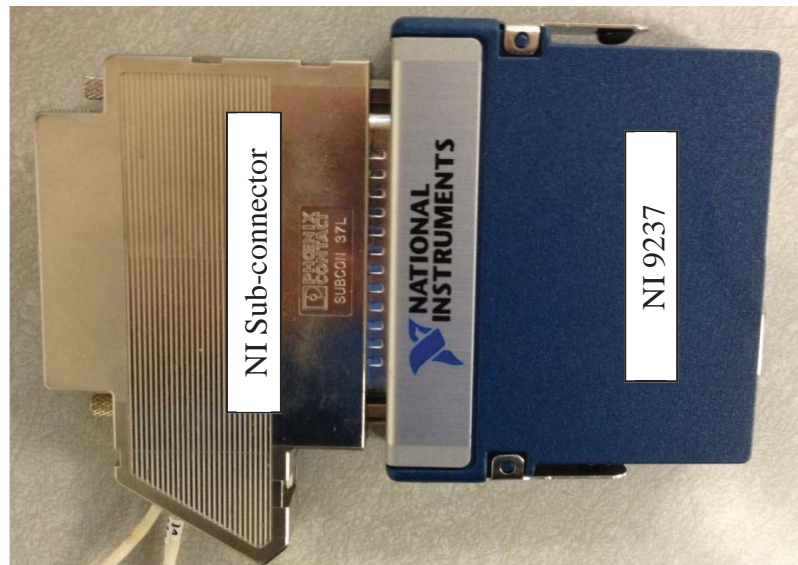
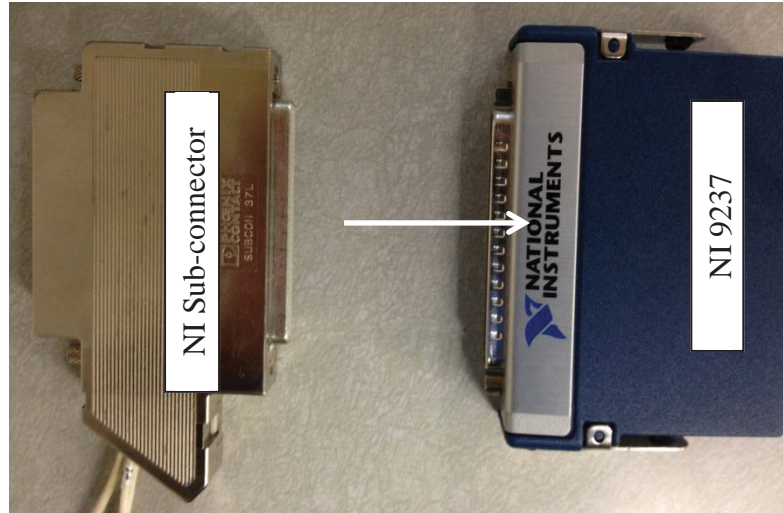


Figure B.3: Wiring diagram for pore pressure transducers in a fully active four arm Wheatstone bridge configuration

It is important to note that the ground wires for each of the three Kulite XCL-11-250-150SG sealed gauge pore pressure transducers were connected to the negative excitation input pin due to grounding issues. A voltmeter was used to check that there was no significant voltage difference between the ground wire and the negative excitation input pin.

Once the wires are correctly connected to the NI sub-connector, insert the NI Sub-connector into the NI 9237 Module as seen in Figures B.4 and B.5.



Figures B.4 (Top) and B.5 (Bottom): Show the connection of the NI sub-connector to the NI 9237 Module

Once the NI sub-connector is inserted into the NI 9237 Module which is then inserted into the NI chassis like Figure 1 shows, it is time to start acquiring data through National Instruments LabVIEW 2012 software.

LabVIEW Program Set-Up

Now it is time to install National Instruments LabVIEW 2012 software into the computer. The LabVIEW 2012 software comes with two cd-ROMs, both of which need to be installed for full use of the software. Once both cd-ROMs are installed, open up NI MAX to make sure that the modules are being read by the LabVIEW 2012 software, after being connected through the USB port. Once the NI MAX user menu opens up, there will be a box along the left hand side of the window. Now click on Devices and Interfaces. The following window should look like the Figure B.6 if the LabVIEW 2012 software is recognizing the NI chaise and NI 9237 Module correctly.

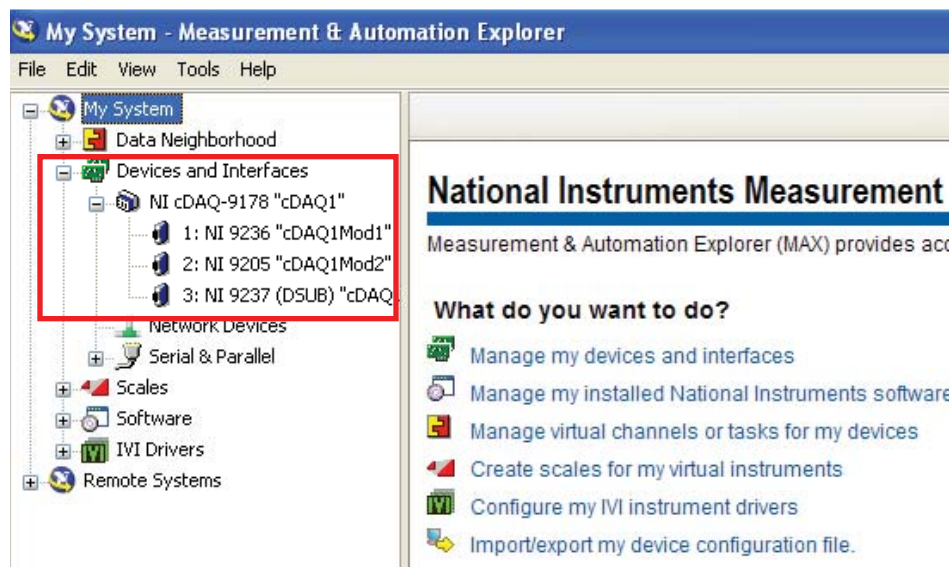


Figure B.6: NI Max user window showing the NI chaise and NI 9237 Module being recognized by LabVIEW 2012.

The next step in getting the data acquisition system working is to open up a brand new VI page in LabVIEW 2012 software. Once you open up a new VI work page, two windows should appear; the first being the Block Diagram, and the second being the

Front Panel. The Block Diagram is where the actually computer code is written and the Front Panel is where all the data is displayed. The figure below shows the two windows that appear when you open up a new VI work page.

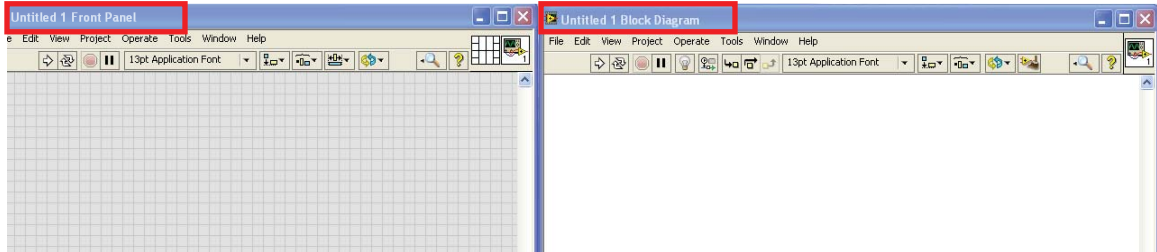


Figure B.7: Shows the Front Panel and the Block Diagram windows once opening up a new VI work page in LabVIEW 2012.

To begin writing code for the data acquisition system, the functions window must be opened in the Block Diagram window. To open the functions window, right click anywhere in the Block Diagram window and click on the push pin in the upper left hand corner of the functions window. Now the functions window should appear and look like the Figure B.8

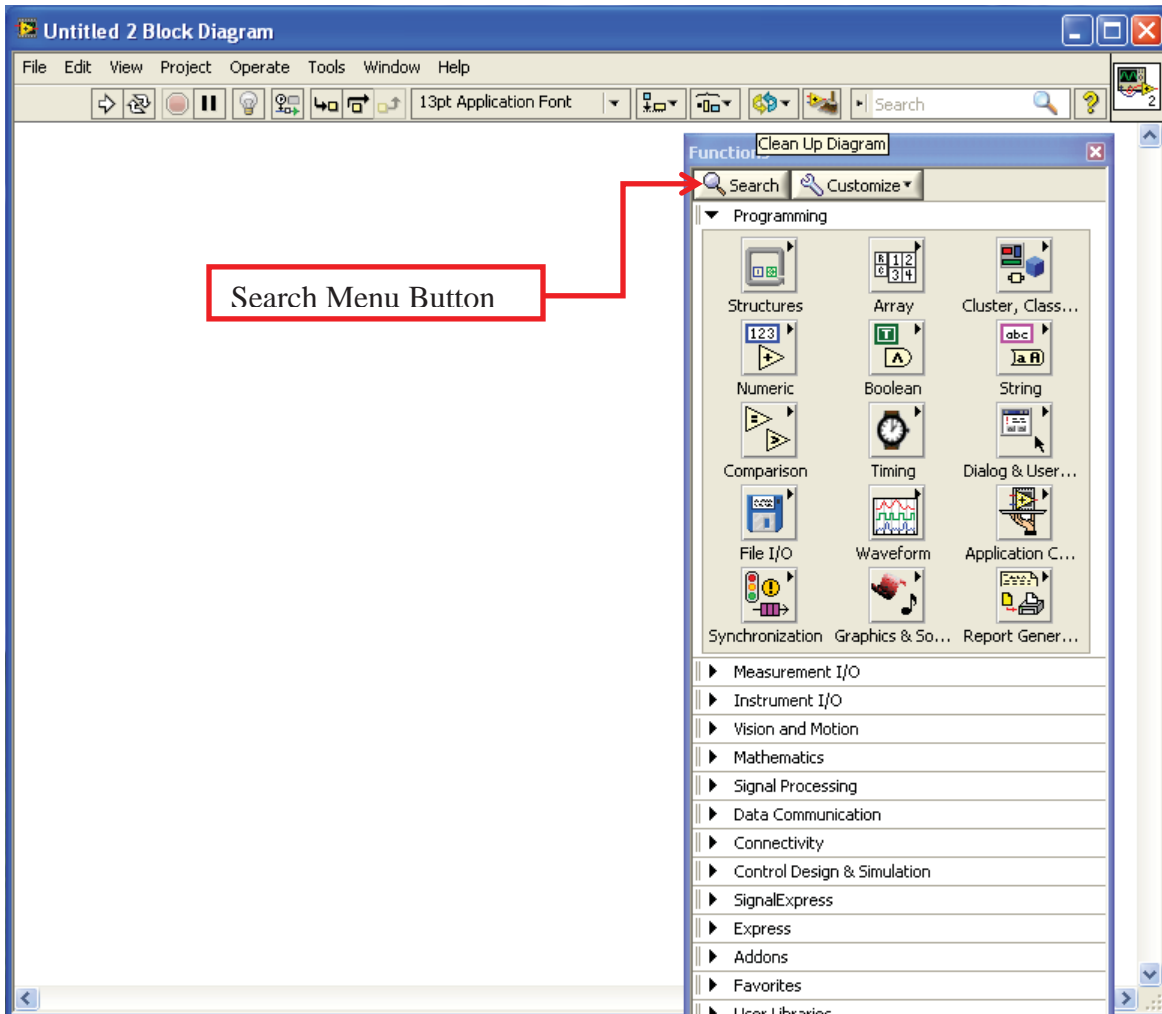


Figure B.8: Shows the functions window opened in the Block Diagram. Also shows the location of the search menu button.

Now that the function window is opened in the Block Diagram, it is time to start configuring functions to acquire pressure data from our three pore pressure transducers. The first function needed is the DAQ Assistant. To find the DAQ Assistant function, click on the “Search” button in the top left corner of the functions and type in “DAQ Assistant”. This can be seen in Figure 8 above. Click and drag the DAQ Assistant Input Icon anywhere on the Block Diagram. A window will appear asking what type of signal it is. Click on Acquire Signals. More options will appear under Acquire Signals. Click

on Analog Input. Once again more options will appear under Analog Input. Click on Pressure then Pressure(Bridge). Now a new window will appear asking for what channels are going to be used. Click on the plus sign to the left of where it shows the NI 9237 Module. This will allow you to see all the different channels within the NI 9237 Module. Since we will only be using three pore pressure transducers, we only need to select channels ai0, ai1, and ai2. Then click the finish button in the bottom of the window. The window for selecting the channels should look like Figure B.9.

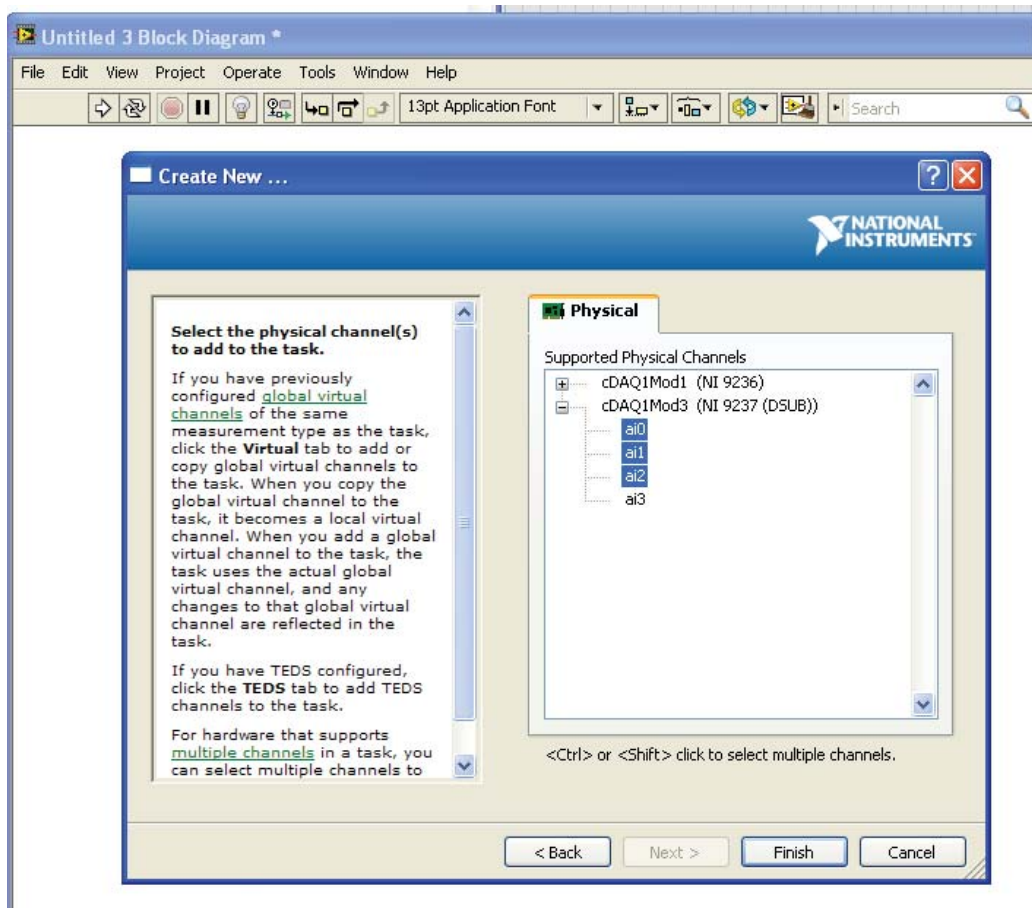


Figure B.9: This figure is showing the three channels that need to be selected to acquire data from the three different pore pressure transducers.

After clicking the finish button, the DAQ Assistant window will appear. On the left hand side of the DAQ Assistant Window, a little box titled "Channel Settings" should appear.

Make sure to highlight all three channels named Pressure_0, Pressure_1, and Pressure_2 so that all three channels can be set up the exact same way. Under the tab “Settings” there are many boxes for user input. Under “Signal Input Range”, the maximum input is 150, the minimum input is 0 and the scaled units are pounds per square inch(psi). The bridge type selected should be Full Bridge and the Vex Source selected should be Internal. The Vex Value is 10 volts and the bridge resistance is 1000 ohms(1k ohms). Select <No Scale> for Custom Scaling. Under “Time Settings” the acquisition mode selected should be Continuous Samples, the Samples to Read should be 1613(1.613k) and the Rate should be 1613 hertz(1.613k). Figure B.10 below shows all the user input under the settings tab in the DAQ Assistant window.

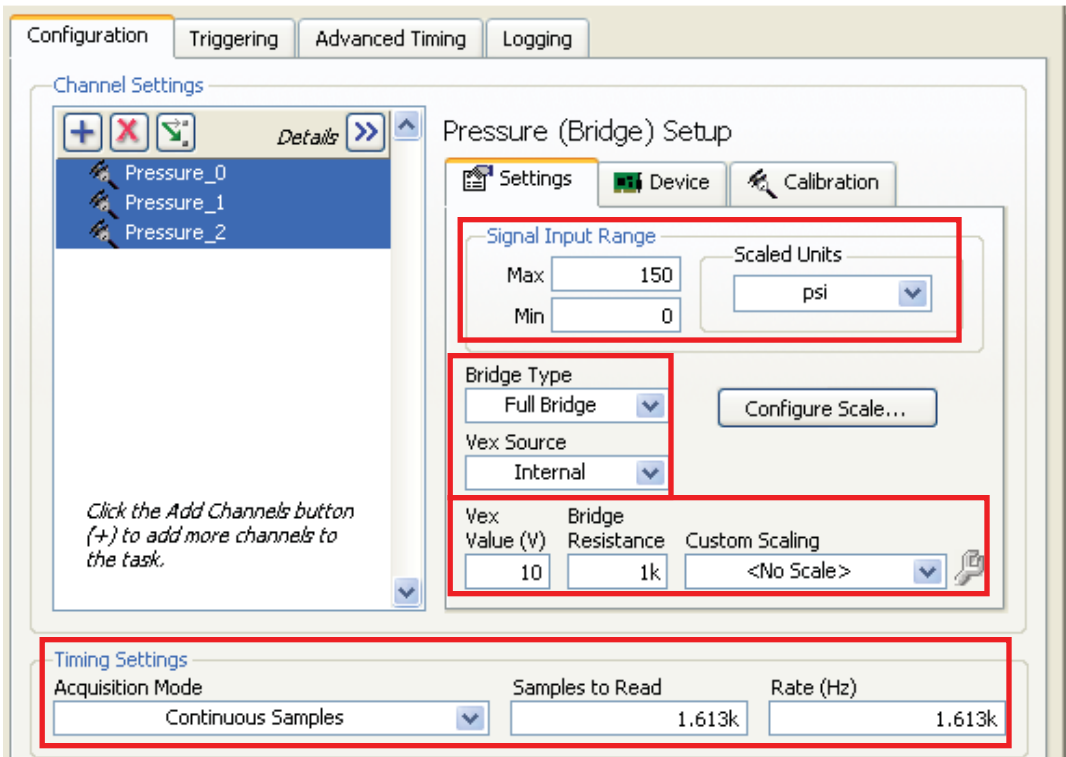


Figure B.10: Shows all the user input that are required for all three channels. By highlighting all three channels at once all the user inputs for all channels will be changed.

After inputting all the user inputs under the settings tab, highlight the first pore pressure transducer (Pressure_0) and click on the “Configure Scale” button which can be seen in Figure B.11.

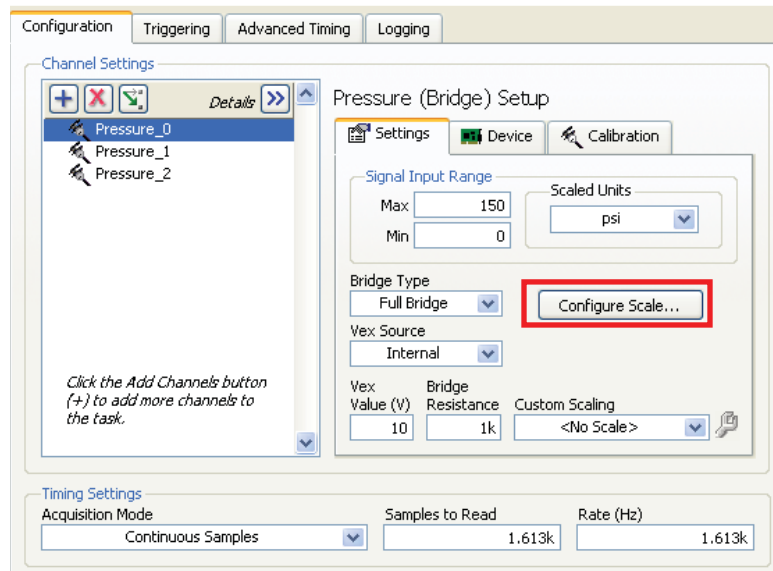


Figure B.11: Shows the location of the configure scale button for pore pressure transducer Pressure_0.

Upon clicking the “Configure Scale” button for the first pore pressure transducer, a new window will appear asking for more user input. Under scale type select “Two-Point Linear”. Under electrical, type in 0 for the “First Value” box and 0.0737 for the “Second Value” box. The 0.0737 value is the sensitivity value that comes from the calibration sheet provided by the pore pressure transducer manufacturer divided by 10. Under physical, type in 0 for the “First Value” box and 1 for the “Second Value” box. Make sure the units under the electrical section is mV/V and units under the physical section are psi. Then click the “OK” button. Figure B.12 shows the exact user input for the first pore pressure transducer.

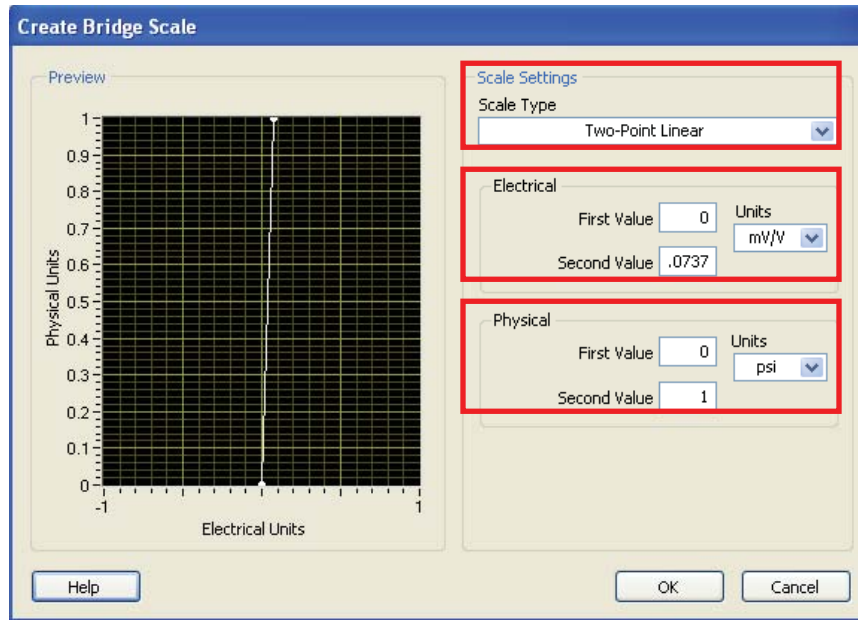


Figure B.12: Shows the user inputs for the first pore pressure transducer(Pressure_0).

Now repeat the same process to configure the scale for the second pore pressure transducer (Pressure_1). Highlight the second pore pressure transducer (Pressure_1) and click on the “Configure Scale” button which can be seen in Figure B.13.

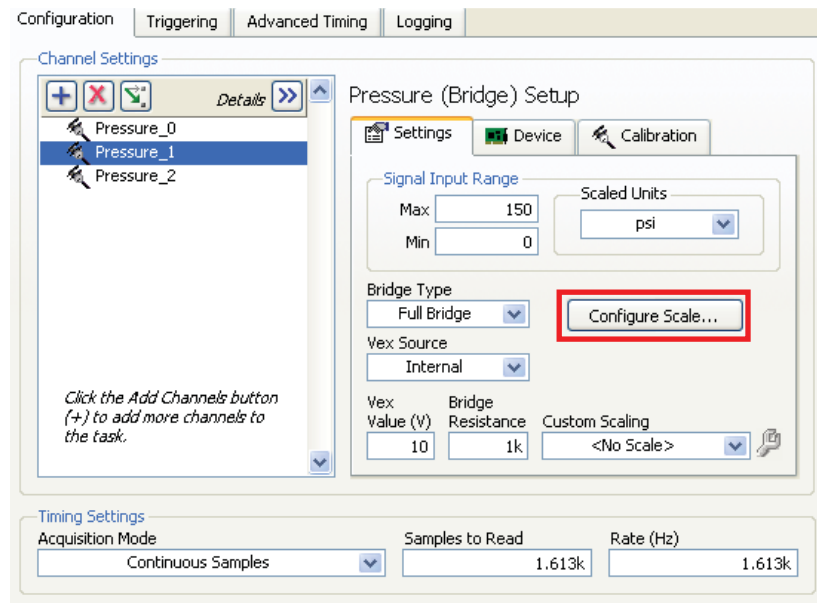


Figure B.13: Shows the location of the configure scale button for pore pressure transducer Pressure_1.

Upon clicking the “Configure Scale” button for the second pore pressure transducer, a new window will appear asking for more user input. Under scale type select “Two-Point Linear”. Under electrical, type in 0 for the “First Value” box and 0.0738 for the “Second Value” box. The 0.0738 value is the sensitivity value that comes from the calibration sheet provided by the pore pressure transducer manufacturer divided by 10. Under physical, type in 0 for the “First Value” box and 1 for the “Second Value” box. Make sure the units under the electrical section is mV/V and units under the physical section are psi. Then click the “OK” button. Figure B.14 shows the exact user input for the second pore pressure transducer.

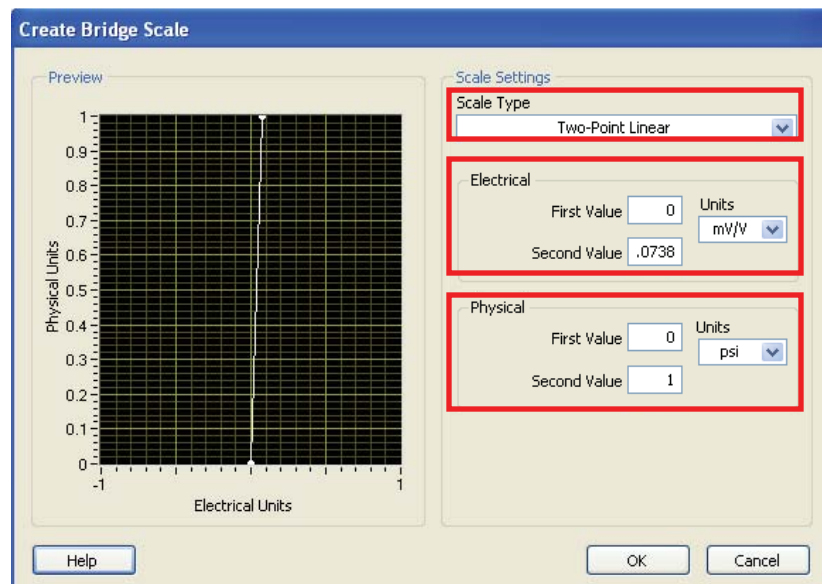


Figure B.14: Shows the user inputs for the second pore pressure transducer (Pressure_1).

Finally repeat this same procedure for the third pore pressure transducer (Pressure_2). Highlight the third pore pressure transducer (Pressure_2) and click on the “Configure Scale” button which can be seen in Figure B.15.

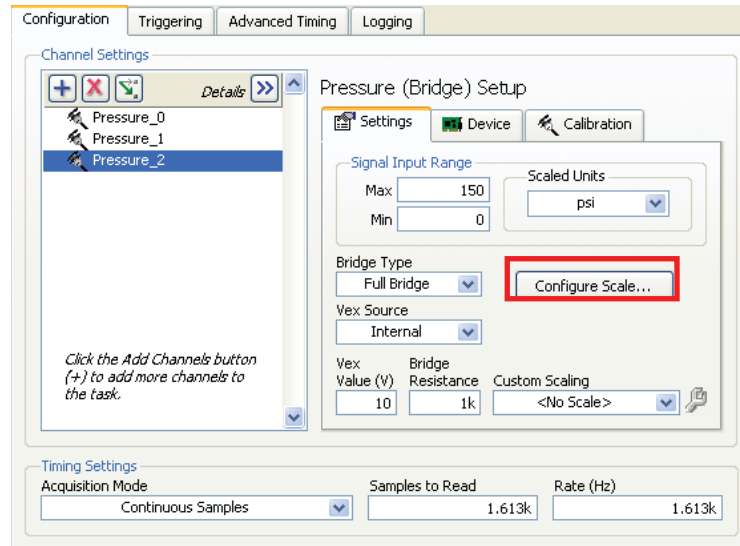


Figure B.15: Shows the location of the configure scale button for pore pressure transducer Pressure_2.

Upon clicking the “Configure Scale” button for the third pore pressure transducer, a new window will appear asking for more user input. Under scale type select “Two-Point Linear”. Under electrical, type in 0 for the “First Value” box and 0.0674 for the “Second Value” box. The 0.0674 value is the sensitivity value that comes from the calibration sheet provided by the pore pressure transducer manufacturer divided by 10. Under physical, type in 0 for the “First Value” box and 1 for the “Second Value” box. Make sure the units under the electrical section is mV/V and units under the physical section are psi. Then click the “OK” button. Figure B.16 shows the exact user input for the third pore pressure transducer.

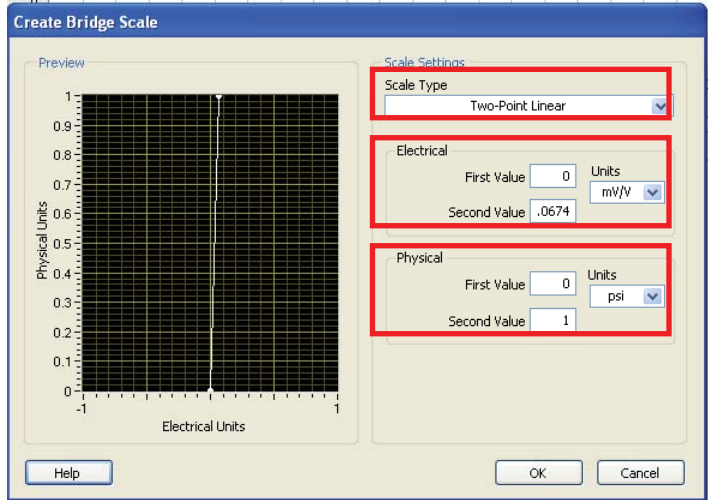


Figure B.16: Shows the user inputs for the third pore pressure transducer (Pressure_2).

Initial Pore Pressure Calibration

Now that all the settings are correct for all three pore pressure transducers the next step in this process is to calibrate all three pore pressure transducers at once. To make sure all three pore pressure transducers get calibrated at the same time, make sure all three channels are selected in the channel settings like in Figure B.17. Then click on the “Calibration” tab inside the DAQ Assistant window. After clicking on the “Calibration” tab the window you should be seeing should look like the figure below.

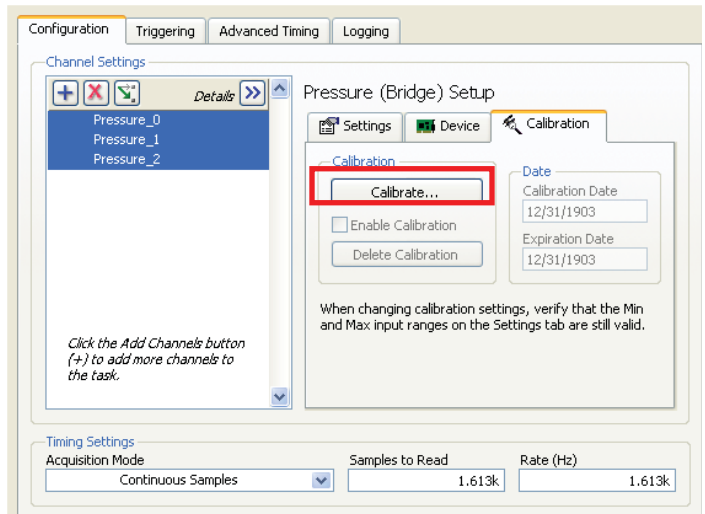


Figure B.17: This figure shows the window that should appear when the “Calibration” tab is clicked within the DAQ Assistant window.

Once this window appears click on the “Calibrate...” button which can be seen in Figure B.17. The user will be asked to type in the calibrator’s name then click the “Next” button. Then the Channel Calibration Wizard Window should appear. NI LabVIEW 2012 will ask for additional user input for the number of samples to average and the sample rate at which the software will acquire data. However, it is important to note that the user should not have to input any information since the NI LabVIEW 2012 software has defaulted values for this criteria. The defaulted values for the number of samples to average and the sample rate both are 25000. Figure B.18 shows all the user input, in our case the defaulted values in the Channel Calibration Wizard window.

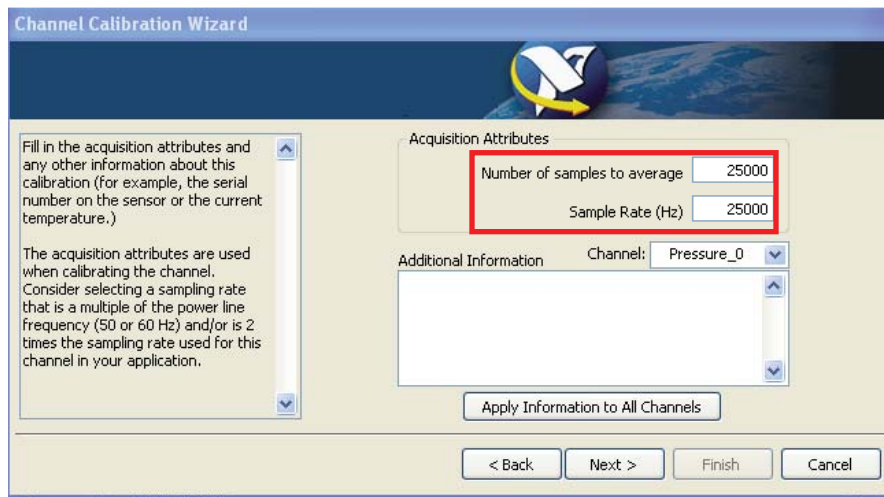


Figure B.18: This figure shows all the user input for the Channel Calibration Wizard window.

After all the user input is in the Channel Calibration Wizard window click the “Next” button to proceed in the calibration process. Now a new user panel within the Channel Calibration Wizard window will appear. There will be a table that will appear in this new

user panel. There should be three columns, a Reference column, an Uncalibrated column and a Difference column. You will be able to see this table in Figure B.19. The Reference column is the only column that user input is required.

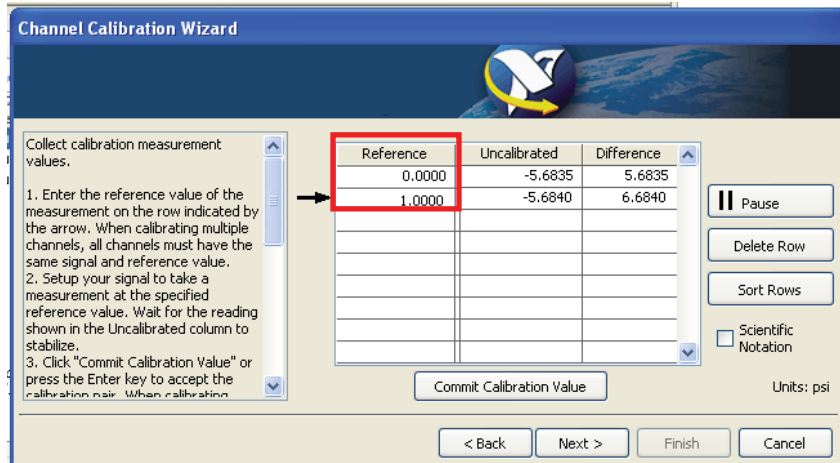


Figure B.19: Shows the values that should be inserted into the Reference Column of the Channel Calibration Wizard window.

Before proceeding any further with the calibration of these three pore pressure transducers within the NI LabVIEW software, one should find a tall narrow cylinder that is approximately 32 inches tall. This cylinder will be used to help in the calibration of these pore pressure transducers. Put a thin piece of tape along the length of the cylinder. Now make marks on this piece of tape that correspond to 0.1 psi increments starting at 0 psi and ending at 1.0 psi. To find the correct heights for different pressure heads use the following equation:

$$\sigma = \gamma H \tag{B.1}$$

Where $\sigma = \text{pressure}$, $\gamma = \text{unit weight of water}$, and $H = \text{height}$

Once all the pressure increments are marked on the calibration test tube, the calibration test tube should look similar to Figure B.20.



Figure B.20: Shows the bottom half of the calibration test cylinder. Also shows the incremental pressure markings along the outside of the calibration test cylinder.

Once all the pressure increments are marked on the calibration test tube, continue with the calibration process within the NI LabVIEW 2012 software. User input is only required in the Reference column of the Channel Calibration Wizard window. Once all three pore pressure are correctly wired like Figure 3 to the NI sub-connector, and the NI sub-connector is plugged into the NI 9237 Module like Figure 5 and the NI 9237 Module is inserted into the correct location of the NI chaise seen in Figure 1, place all three pore pressure transducers at the bottom of the calibration test tube where the test tube marking should indicate 0 psi. In the first row of the Reference column insert the value 0. The Reference column is the so called “theoretical” pressure values that the pore pressure transducers should be reading. The Uncalibrated column shows the raw data values that are coming from the pore pressure transducers. Once the Reference value for the 0 psi marking is inputted, click the “Commit Calibration Value” button at the bottom of the window. Now fill the calibration test tube up with room temperature water to the 1.0 psi

marking. In the second row of the Reference column type in the number 1, then click the “Commit Calibration Value” button at the bottom of the window. Click the “Next” button until you see a window that shows a “Finish” button. Once you see the finish button at the bottom of the window click it. Then inside the DAQ Assistant window click the “OK” button in the bottom right hand corner. The Block Diagram should now look similar to Figure B.21.

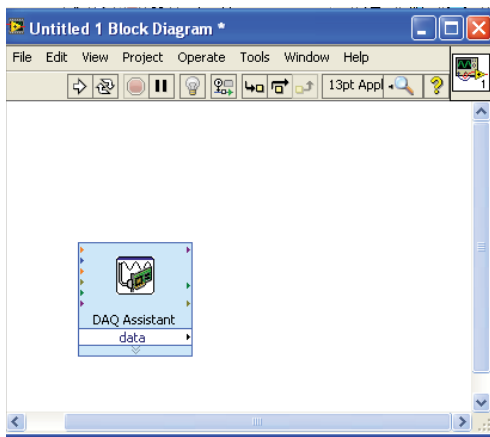


Figure B.21: Shows the DAQ Assistant function icon on the block diagram after being calibrated.

Once the DAQ Assistant function is in the Block Diagram, the next function to insert into the block diagram is the Sample Compression function. To insert the Sample Compression function, go back to the functions tool bar, click on the “Search” button and type in “Sample Compression”. When the icon for the function appears, drag and drop the function anywhere in the Block Diagram window. A window will appear after inserting the Sample Compression function. The window that pops up should look like Figure 22. The only user input in this window is the reduction factor. The reduction factor should be set to 1613 and the reduction method should be mean. After these user

inputs are inserted click the “OK” button. The outlined red boxes in Figure B.22 show the user where all the user inputs are located for the Sample Compression function.

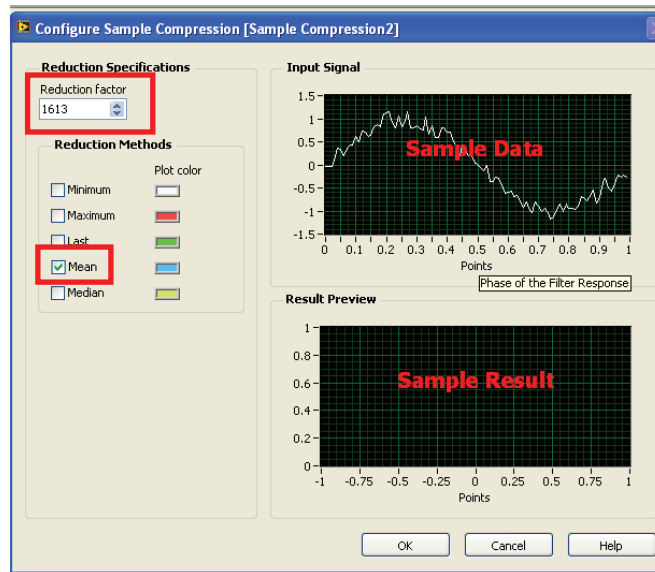


Figure B.22: Shows the window that pops up after inserting the Sample Compression function into the block diagram. The red boxes indicate where user input is required.

The next function to be inserted into the Block Diagram is the Dynamic Data Type function. To insert the Dynamic Data Type function, go back to the functions tool bar, click on the “Search” button and type in “Dynamic Data Type”. When the icon for the function appears, drag and drop the function anywhere in the Block Diagram window. A window will appear after inserting the Dynamic Data Type function and should look similar to Figure B.23. Under resulting data type, select “2D array of scalars-columns are channels”. Also, under Scalar Data Type select “Floating point numbers (double)”. The red outlined boxes show where user inputs are required.

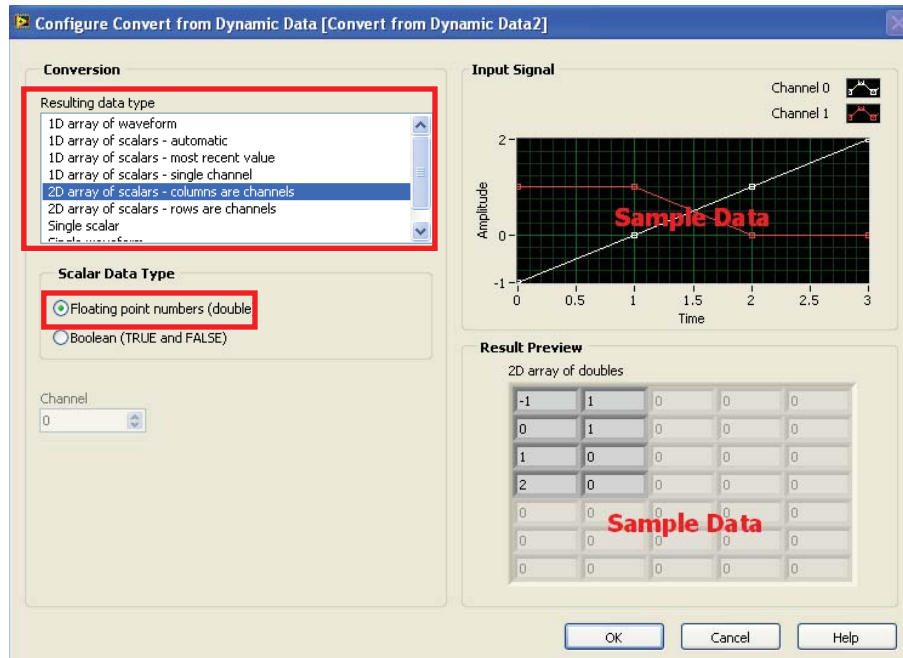


Figure B.23: Shows the window that pops up after inserting the Dynamic Data Type function into the block diagram. The red boxes indicate where user input is required.

Now insert the Insert Into Array function into the Block Diagram. Go back to the functions tool bar, click on the “Search” button and type in “Insert Into Array”. When the icon for the function appears, drag and drop the function anywhere in the Block Diagram window. Do the exact same procedure but for the Output Array. Once all the functions are in the Block Diagram enclose the functions within a while loop so continuous measurements can be taken.

Once all the functions are within the while loop in the Block Diagram, take a look at Figure B.24 to see the correct wiring connections between functions.

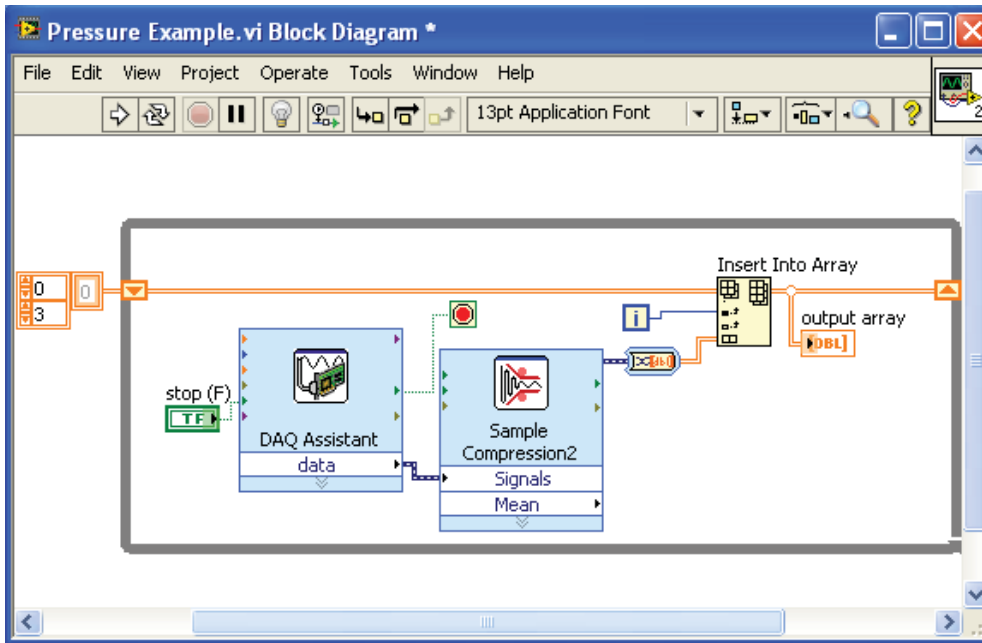


Figure B.24: Shows the correct wiring connections between functions within the Block Diagram.

Once all the connections are made similar to Figure B.24, it's time to start acquiring data from the three pore pressure transducers. By hitting the "Run" button on the Block Diagram, a table of numbers will appear which should look similar to Figure B.25. The first column shows pressure values coming from the pore pressure transducer in channel ai0, the second column shows pressure values coming from the pore pressure transducer in channel ai1, and the third column shows pressure values coming from the pore pressure transducer in channel ai2.

| | Pressure_0 | Pressure_1 | Pressure_2 |
|---|------------|------------|------------|
| 0 | 0.228599 | 0.445427 | 0.876372 |
| 0 | 0.228622 | 0.445428 | 0.876805 |
| | 0.228493 | 0.445357 | 0.87638 |
| | 0.228309 | 0.445419 | 0.877293 |
| | 0.228349 | 0.44499 | 0.877179 |
| | 0.228163 | 0.445032 | 0.875683 |
| | 0.228446 | 0.445141 | 0.875325 |
| | 0.228648 | 0.445245 | 0.875689 |
| | 0.228832 | 0.445143 | 0.875852 |

Figure B.25: Shows the table that will appear after hitting the “Run” button in the Block Diagram. This table will appear in the Front Panel.

Secondary Calibration

Once values are appearing in the table, it’s time to verify that these readings from the pore pressure transducers are in fact reading the correct pressure. First secure all three pore pressure transducers with a piece of tape to the bottom of the calibration test tube. Next start filling up the calibration test tube with water, to levels that correspond to the different 0.1 psi incremental markings. First start with 0.1 psi and start working your way up until you reach 1 psi. All three pore pressure transducers should be reading similar numbers at every 0.1 psi increment. If this is not the case, a secondary calibration must be performed to verify that the readings are accurate. The secondary calibration is similar to the first calibration that we performed in NI LabVIEW 2012. The first step in the secondary calibration is to make sure all the pore pressure transducers are at the same moisture content. To make sure all three pore pressure transducers are at the same moisture content, dip them into water for approximately one minute. Once the pore pressure transducers have been sitting in water for one minute, let all three pore pressure transducers air dry for 24-hours. After all three pore pressure transducers are at the same

moisture content, secure them to the bottom of the calibration test tube with a piece of tape. Then fill the calibration test tube with water up to the 0.4 psi marking. Run the code in NI LabVIEW 2012 and pick a number from each column that is representative of the pressure readings for each pore pressure transducer. Formulate three tables, one for each pore pressure transducer in Microsoft Excel. All three tables should have a column for LabVIEW pressure readings and a column for theoretical pressure values. The theoretical pressure values are the markings on the side of calibration test tube. Once you have picked a representative number for each pore pressure transducer at the 0.4 psi marking on the calibration test tube, insert values into your Excel table under the LabVIEW pressure readings column. Continue the above procedure at every 0.2 psi marking starting at 0.4 psi and ending at 1.0 psi on the calibration test tube. Thus, you should have a total of 5 different LabVIEW pressure readings and 5 different theoretical pressure values for each pore pressure transducer. Now, plot LabVIEW pressure readings versus theoretical pressure values for all three pore pressure transducers. Insert a linear trendline and display the equation of the trendline on the graph. Each pore pressure transducer will have a separate linear equation.

Now, the raw values that come from LabVIEW have to run through the specific equation for that pore pressure transducer that came from the secondary calibration. Once the secondary calibration is complete, nest the three different pore pressure transducers at different pressure markings inside the calibration test tube. Run the NI LabVIEW software, grab representative pressure readings for all three pore pressure transducers and then calculate the new pressure values by using the equations from the secondary calibration. These new pressure readings should match up to the pressure

markings on the calibration test tube where the pore pressure transducers are nested. The pore pressure transducers are now calibrated and ready to be used for application.

Appendix C:

Raw Data

| Spec ID | # of POCs | Pore Pressure Gauge Pressure - Ave Pressure (kPa) | Avg Pore Pressure (kPa) | Normalised Pore Pressure (kPa) | Normalised Ave Pressure (kPa) | Normalised Pore Pressure (kPa) | Normalised Ave Pressure (kPa) | Sds | PC Side | | PC Pipe Array at Secondary Trenching | | PC Pipe Array at Secondary Spacing of 1500 mm | | Length (m) | Time (min) | Flow Rate (m³/sec) | Normalised Flow Rate | Logon Value | Normalised Average Logon Value | Logon Value Gauge 13 | Logon Value Gauge 12 | Logon Value Gauge 11 |
|---------|-----------|---|-------------------------|--------------------------------|-------------------------------|--------------------------------|-------------------------------|-----------|-------------------------------|--------------------------------|--------------------------------------|--------------------------------|---|--------------------------------|------------|------------|--------------------|----------------------|-------------|--------------------------------|----------------------|----------------------|----------------------|
| | | | | | | | | | Normalised Ave Pressure (kPa) | Normalised Pore Pressure (kPa) | Normalised Ave Pressure (kPa) | Normalised Pore Pressure (kPa) | Normalised Ave Pressure (kPa) | Normalised Pore Pressure (kPa) | | | | | | | | | |
| Bar1 | 0 | 5.10 | 5.10 | 5.22 | 5.20 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 573 | 5 | 0.46 | 0.4633305 | 1 | 51.07636 | 1 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 3 | 5.13 | 5.14 | 5.10 | 5.10 | 0.959707 | 0.966680 | 0.1111111 | 0.1344703 | 0.0726643 | 0.1344703 | 0.0483201 | 0.0344703 | 571 | 5 | 0.46 | 0.4444703 | 0.0565501 | 51.07636 | 0.0565501 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 4 | 5.10 | 5.10 | 5.10 | 5.10 | 0.9706270 | 0.9806270 | 0.1048408 | 0.1281940 | 0.1075203 | 0.1281940 | 0.0781940 | 0.0781940 | 574 | 5 | 0.46 | 0.4523207 | 0.0491940 | 51.07636 | 0.0491940 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 5 | 5.10 | 5.10 | 5.02 | 5.00 | 0.9792020 | 0.9792020 | 0.1036205 | 0.1279304 | 0.1044404 | 0.1279304 | 0.0772202 | 0.0779304 | 544 | 5 | 0.46 | 0.4532304 | 0.0500205 | 51.07636 | 0.0500205 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 6 | 4.81 | 4.81 | 4.82 | 4.86 | 0.9575207 | 0.9586653 | 0.1022202 | 0.1269205 | 0.1044404 | 0.1269205 | 0.0772202 | 0.0779304 | 515 | 5 | 0.46 | 0.4488205 | 0.0510207 | 51.07636 | 0.0510207 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 7 | 4.54 | 4.62 | 4.63 | 4.68 | 0.871308 | 0.8666203 | 0.1029209 | 0.1244706 | 0.1044404 | 0.1244706 | 0.0772202 | 0.0779304 | 470 | 5 | 0.46 | 0.4055207 | 0.0442709 | 51.07636 | 0.0442709 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 8 | 4.88 | 4.70 | 4.70 | 4.70 | 0.8770705 | 0.8454209 | 0.1044404 | 0.1244706 | 0.1044404 | 0.1244706 | 0.0772202 | 0.0779304 | 463 | 5 | 0.46 | 0.4088207 | 0.0442709 | 51.07636 | 0.0442709 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 9 | 4.64 | 4.73 | 4.69 | 4.73 | 0.8036717 | 0.8054209 | 0.1033303 | 0.1234408 | 0.1044404 | 0.1234408 | 0.0772202 | 0.0779304 | 400 | 5 | 0.46 | 0.3793303 | 0.0419107 | 51.07636 | 0.0419107 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 10 | 3.93 | 3.92 | 3.95 | 3.93 | 0.7092705 | 0.7054209 | 0.1029209 | 0.1234408 | 0.1044404 | 0.1234408 | 0.0772202 | 0.0779304 | 384 | 5 | 0.46 | 0.3544209 | 0.0387005 | 51.07636 | 0.0387005 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 11 | 3.24 | 3.20 | 3.20 | 3.21 | 0.7062709 | 0.7054209 | 0.1044404 | 0.1234408 | 0.1044404 | 0.1234408 | 0.0772202 | 0.0779304 | 370 | 5 | 0.46 | 0.3204209 | 0.0377002 | 51.07636 | 0.0377002 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 12 | 3.90 | 3.87 | 3.87 | 3.86 | 0.6924209 | 0.6924209 | 0.1044404 | 0.1234408 | 0.1044404 | 0.1234408 | 0.0772202 | 0.0779304 | 355 | 5 | 0.46 | 0.3573209 | 0.0370907 | 51.07636 | 0.0370907 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 13 | 3.91 | 3.92 | 3.91 | 3.91 | 0.6786204 | 0.6786204 | 0.1044404 | 0.1234408 | 0.1044404 | 0.1234408 | 0.0772202 | 0.0779304 | 340 | 5 | 0.46 | 0.3273209 | 0.0352508 | 51.07636 | 0.0352508 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 14 | 3.42 | 3.34 | 3.31 | 3.29 | 0.5271096 | 0.5271096 | 0.1044404 | 0.1234408 | 0.1044404 | 0.1234408 | 0.0772202 | 0.0779304 | 310 | 5 | 0.46 | 0.2742020 | 0.0345208 | 51.07636 | 0.0345208 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 15 | 3.42 | 3.40 | 3.40 | 3.40 | 0.3661073 | 0.3661073 | 0.1044404 | 0.1234408 | 0.1044404 | 0.1234408 | 0.0772202 | 0.0779304 | 295 | 5 | 0.46 | 0.2364409 | 0.0336604 | 51.07636 | 0.0336604 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 16 | 2.64 | 2.63 | 2.63 | 2.63 | 0.5454209 | 0.5454209 | 0.1044404 | 0.1234408 | 0.1044404 | 0.1234408 | 0.0772202 | 0.0779304 | 271 | 5 | 0.46 | 0.1573209 | 0.0325208 | 51.07636 | 0.0325208 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 17 | 2.49 | 2.46 | 2.52 | 2.52 | 0.4025406 | 0.4025406 | 0.1044404 | 0.1234408 | 0.1044404 | 0.1234408 | 0.0772202 | 0.0779304 | 248 | 5 | 0.46 | 0.2473209 | 0.0325208 | 51.07636 | 0.0325208 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 18 | 2.10 | 2.10 | 2.09 | 2.12 | 0.4125206 | 0.3946204 | 0.1044404 | 0.1234408 | 0.1044404 | 0.1234408 | 0.0772202 | 0.0779304 | 195 | 5 | 0.46 | 0.1542020 | 0.0325208 | 51.07636 | 0.0325208 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 19 | 1.83 | 1.86 | 1.86 | 1.86 | 0.3944407 | 0.3944407 | 0.1044404 | 0.1234408 | 0.1044404 | 0.1234408 | 0.0772202 | 0.0779304 | 188 | 5 | 0.46 | 0.1484206 | 0.0324407 | 51.07636 | 0.0324407 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 20 | 1.24 | 1.26 | 1.25 | 1.24 | 0.3446204 | 0.3446204 | 0.1044404 | 0.1234408 | 0.1044404 | 0.1234408 | 0.0772202 | 0.0779304 | 180 | 5 | 0.46 | 0.1384206 | 0.0324407 | 51.07636 | 0.0324407 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 21 | 1.63 | 1.64 | 1.65 | 1.64 | 0.3446204 | 0.3446204 | 0.1044404 | 0.1234408 | 0.1044404 | 0.1234408 | 0.0772202 | 0.0779304 | 180 | 5 | 0.46 | 0.1384206 | 0.0324407 | 51.07636 | 0.0324407 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 22 | 1.24 | 1.61 | 1.61 | 1.64 | 0.3446204 | 0.3446204 | 0.1044404 | 0.1234408 | 0.1044404 | 0.1234408 | 0.0772202 | 0.0779304 | 195 | 5 | 0.46 | 0.1384206 | 0.0324407 | 51.07636 | 0.0324407 | 51.07636 | 51.07636 | 51.07636 |
| Bar1 | 23 | 1.50 | 1.51 | 1.55 | 1.59 | 0.2864407 | 0.2864407 | 0.1044404 | 0.1234408 | 0.1044404 | 0.1234408 | 0.0772202 | 0.0779304 | 142 | 5 | 0.46 | 0.1274206 | 0.0324407 | 51.07636 | 0.0324407 | 51.07636 | 51.07636 | 51.07636 |

Figure C.2: Raw Data from S/D=1.18, Unit weight= 12.56 kN/m³

| SPL-6 | PFC (S) (4) (5) | | S4S | | PFC (S) (4) (5) | | PFC (S) (4) (5) | | PFC (S) (4) (5) | | PFC (S) (4) (5) | | Length (ft) | Time (min) | Length (in) | PFC (S) (4) (5) | Normalized Average Uplift Value | Uplift Value | Normalized Average Uplift Value | Uplift Value | Uplift Value | Uplift Value |
|--------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|-------------|------------|-------------|-----------------|---------------------------------|--------------|---------------------------------|--------------|--------------|--------------|
| | Normalized Avg. PFC Pressure (PSF) | Normalized Avg. PFC Pressure (PSF) | Normalized Avg. PFC Pressure (PSF) | Normalized Avg. PFC Pressure (PSF) | Normalized Avg. PFC Pressure (PSF) | Normalized Avg. PFC Pressure (PSF) | Normalized Avg. PFC Pressure (PSF) | Normalized Avg. PFC Pressure (PSF) | Normalized Avg. PFC Pressure (PSF) | Normalized Avg. PFC Pressure (PSF) | Normalized Avg. PFC Pressure (PSF) | Normalized Avg. PFC Pressure (PSF) | | | | | | | | | | |
| Run 1 | 5.24 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| Run 2 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| Run 3 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| Run 4 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| Run 5 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| Run 6 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| Run 7 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| Run 8 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| Run 9 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| Run 10 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| Run 11 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| Run 12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| Run 13 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| Run 14 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| Run 15 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| Run 16 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| Run 17 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| Run 18 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| Run 19 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |

Figure C.3: Raw Data from S/D=1.45, Unit weight= 12.56 kN/m³

| S/D=1.62 | Port Pressure Gauge | Per Pressure Gauge | Per Pressure Gauge | Avg. Pressure | Normalized Pressure | Normalized Pressure | Normalized Pressure | S/Ds | PVC Pipe Primary and Secondary Tranching | | PVC Pipe Primary and Secondary Splicing of | | Time (min) | Length (m) | Flow Loss | Normalized Flow Loss | Legion Value | Normalized Average Legion Value | Legion Value | Legion Value | Legion Value |
|----------|---------------------|--------------------|--------------------|---------------|---------------------|---------------------|---------------------|-----------|--|-----------------------------------|--|-----------------------------------|------------|------------|-----------|----------------------|--------------|---------------------------------|--------------|--------------|--------------|
| | | | | | | | | | Normalized Area Replacement Ratio | Normalized Area Replacement Ratio | Normalized Area Replacement Ratio | Normalized Area Replacement Ratio | | | | | | | | | |
| Run 1 | 0 | 5.23 | 5.23 | 5.38 | 5.24 | 1 | 1 | 0 | 0 | 0 | 0 | 5.42 | 35 | 0.46 | 0.4888955 | 1 | 51.767442 | 51.767442 | 51.767442 | 51.767442 | 51.767442 |
| Run 2 | 3 | 5.28 | 5.23 | 5.24 | 5.28 | 0.9948039 | 1.0080536 | 0.1111111 | 0.1740288 | 0.0832321 | 0.1740288 | 0.0902835 | 5.45 | 35 | 0.46 | 0.4811405 | 1.0053158 | 51.557289 | 51.557289 | 51.557289 | 51.557289 |
| Run 3 | 4 | 5.19 | 5.21 | 5.12 | 5.18 | 0.9597953 | 0.9489975 | 0.1481848 | 0.2524118 | 0.0817942 | 0.2524118 | 0.0954513 | 5.59 | 35 | 0.46 | 0.4853206 | 0.9484161 | 51.570673 | 51.570673 | 51.570673 | 51.570673 |
| Run 4 | 5 | 5.18 | 5.13 | 5.12 | 5.14 | 0.939815 | 0.9732029 | 0.1854548 | 0.2411797 | 0.0727262 | 0.2411797 | 0.0817942 | 5.59 | 35 | 0.46 | 0.4833524 | 0.9874023 | 51.632679 | 51.632679 | 51.632679 | 51.632679 |
| Run 5 | 6 | 4.94 | 4.91 | 4.97 | 4.91 | 0.9492721 | 0.9300796 | 0.2202202 | 0.3294176 | 0.0729463 | 0.3294176 | 0.0817942 | 5.59 | 35 | 0.46 | 0.4517913 | 0.9246811 | 51.038622 | 51.038622 | 51.038622 | 51.038622 |
| Run 6 | 7 | 4.12 | 4.57 | 4.21 | 4.01 | 0.8611239 | 0.8093584 | 0.2825259 | 0.4174706 | 0.0380373 | 0.4174706 | 0.0844693 | 4.95 | 35 | 0.46 | 0.4291264 | 0.8672153 | 51.153396 | 51.153396 | 51.153396 | 51.153396 |
| Run 7 | 8 | 4.59 | 4.84 | 4.6 | 4.61 | 0.8819258 | 0.8458436 | 0.2426256 | 0.3388126 | 0.0380373 | 0.3388126 | 0.0380373 | 4.98 | 35 | 0.46 | 0.42 | 0.8164843 | 51.26571 | 51.26571 | 51.26571 | 51.26571 |
| Run 8 | 9 | 4.92 | 4.61 | 4.51 | 4.65 | 0.8459111 | 0.8303702 | 0.1813103 | 0.2401136 | 0.0380373 | 0.2401136 | 0.0380373 | 4.98 | 35 | 0.46 | 0.4612136 | 0.8368317 | 51.26571 | 51.26571 | 51.26571 | 51.26571 |
| Run 9 | 10 | 4.13 | 4.13 | 4.13 | 4.13 | 0.8141485 | 0.7949492 | 0.1471427 | 0.2149239 | 0.0380373 | 0.2149239 | 0.0380373 | 4.98 | 35 | 0.46 | 0.4554402 | 0.8368317 | 51.26571 | 51.26571 | 51.26571 | 51.26571 |
| Run 10 | 11 | 4.13 | 3.81 | 4.29 | 4.03 | 0.7711485 | 0.7492664 | 0.1471427 | 0.2149239 | 0.0380373 | 0.2149239 | 0.0380373 | 4.98 | 35 | 0.46 | 0.4554402 | 0.8368317 | 51.26571 | 51.26571 | 51.26571 | 51.26571 |
| Run 11 | 12 | 3.76 | 3.98 | 3.87 | 3.80 | 0.7492664 | 0.7492664 | 0.1471427 | 0.2149239 | 0.0380373 | 0.2149239 | 0.0380373 | 3.88 | 35 | 0.46 | 0.3882027 | 0.6292152 | 50.002523 | 50.002523 | 50.002523 | 50.002523 |
| Run 12 | 13 | 3.78 | 3.73 | 3.73 | 3.78 | 0.7492664 | 0.7492664 | 0.1471427 | 0.2149239 | 0.0380373 | 0.2149239 | 0.0380373 | 3.88 | 35 | 0.46 | 0.3882027 | 0.6292152 | 50.002523 | 50.002523 | 50.002523 | 50.002523 |
| Run 13 | 14 | 3.73 | 3.62 | 3.61 | 3.68 | 0.6848108 | 0.6202940 | 0.1471427 | 0.2149239 | 0.0380373 | 0.2149239 | 0.0380373 | 3.88 | 35 | 0.46 | 0.3882027 | 0.6292152 | 50.002523 | 50.002523 | 50.002523 | 50.002523 |
| Run 14 | 15 | 3.41 | 3.49 | 3.43 | 3.48 | 0.6848108 | 0.6202940 | 0.1471427 | 0.2149239 | 0.0380373 | 0.2149239 | 0.0380373 | 3.88 | 35 | 0.46 | 0.3882027 | 0.6292152 | 50.002523 | 50.002523 | 50.002523 | 50.002523 |
| Run 15 | 16 | 3.14 | 3.19 | 3.13 | 3.18 | 0.6202940 | 0.5862497 | 0.1471427 | 0.2149239 | 0.0380373 | 0.2149239 | 0.0380373 | 3.88 | 35 | 0.46 | 0.3882027 | 0.6292152 | 50.002523 | 50.002523 | 50.002523 | 50.002523 |
| Run 15 | 17 | 3.07 | 3.01 | 3.06 | 3.07 | 0.5862497 | 0.5520206 | 0.1471427 | 0.2149239 | 0.0380373 | 0.2149239 | 0.0380373 | 3.88 | 35 | 0.46 | 0.3882027 | 0.6292152 | 50.002523 | 50.002523 | 50.002523 | 50.002523 |

Figure C.4: Raw Data from S/D=1.62, Unit weight=12.56 kN/m³

| SPOUT | # of PVC Pipe | Pvc Pressure Comp. (MPa) | Avg Pvc Pressure (MPa) | Normalized Pvc Pressure Change (MPa) | Normalized Pvc Pressure Change (MPa) | Normalized Pvc Pressure Change (MPa) | Normalized Pvc Pressure Change (MPa) | Sides | PVC S44 Side | | PVC Pipe-Primary and Secondary Flowing | | PVC Pipe-Primary and Secondary Spacing of 1.57m of Pipe | | Time (min) | Length (m) | Flow Loss (Day/Time/Length) (m/pipe/hr) | Normalized Flow Loss | Lupen Value | Normalized Average Lupen Value | Lupen Value Comp 13 | Lupen Value Comp 23 | Lupen Value Comp 34 |
|-------|---------------|--------------------------|------------------------|--------------------------------------|--------------------------------------|--------------------------------------|--------------------------------------|-------|-------------------------------------|--------------------------|--|--------------------------|---|--------------------------|------------|------------|---|----------------------|-------------|--------------------------------|---------------------|---------------------|---------------------|
| | | | | | | | | | Normalized Area Replenishment Ratio | Area Replenishment Ratio | Normalized Area Replenishment Ratio | Area Replenishment Ratio | Normalized Area Replenishment Ratio | Area Replenishment Ratio | | | | | | | | | |
| 0 | 0 | 5.17 | 5.15 | 5.19 | 5.19 | 1 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 253 | 0.6 | 0.452426 | 1 | 94.03375 | 1.0 | 94.04424 | 94.13553 | 93.42487 |
| 1 | 1 | 5.17 | 5.15 | 5.19 | 5.19 | 1 | 1 | 1 | 0.072543 | 0.237021 | 0.083521 | 0.237021 | 0.083521 | 0.237021 | 261 | 0.6 | 0.452426 | 1 | 94.03375 | 1.0 | 94.04424 | 94.13553 | 93.42487 |
| 2 | 2 | 5.17 | 5.15 | 5.19 | 5.19 | 1 | 1 | 1 | 0.072543 | 0.237021 | 0.083521 | 0.237021 | 0.083521 | 0.237021 | 261 | 0.6 | 0.452426 | 1 | 94.03375 | 1.0 | 94.04424 | 94.13553 | 93.42487 |
| 3 | 3 | 5.17 | 5.15 | 5.19 | 5.19 | 1 | 1 | 1 | 0.072543 | 0.237021 | 0.083521 | 0.237021 | 0.083521 | 0.237021 | 261 | 0.6 | 0.452426 | 1 | 94.03375 | 1.0 | 94.04424 | 94.13553 | 93.42487 |
| 4 | 4 | 5.17 | 5.15 | 5.19 | 5.19 | 1 | 1 | 1 | 0.072543 | 0.237021 | 0.083521 | 0.237021 | 0.083521 | 0.237021 | 261 | 0.6 | 0.452426 | 1 | 94.03375 | 1.0 | 94.04424 | 94.13553 | 93.42487 |
| 5 | 5 | 5.17 | 5.15 | 5.19 | 5.19 | 1 | 1 | 1 | 0.072543 | 0.237021 | 0.083521 | 0.237021 | 0.083521 | 0.237021 | 261 | 0.6 | 0.452426 | 1 | 94.03375 | 1.0 | 94.04424 | 94.13553 | 93.42487 |
| 6 | 6 | 4.81 | 4.82 | 4.84 | 4.84 | 1 | 1 | 1 | 0.072543 | 0.237021 | 0.083521 | 0.237021 | 0.083521 | 0.237021 | 261 | 0.6 | 0.452426 | 1 | 94.03375 | 1.0 | 94.04424 | 94.13553 | 93.42487 |
| 7 | 7 | 4.81 | 4.82 | 4.84 | 4.84 | 1 | 1 | 1 | 0.072543 | 0.237021 | 0.083521 | 0.237021 | 0.083521 | 0.237021 | 261 | 0.6 | 0.452426 | 1 | 94.03375 | 1.0 | 94.04424 | 94.13553 | 93.42487 |
| 8 | 8 | 4.81 | 4.82 | 4.84 | 4.84 | 1 | 1 | 1 | 0.072543 | 0.237021 | 0.083521 | 0.237021 | 0.083521 | 0.237021 | 261 | 0.6 | 0.452426 | 1 | 94.03375 | 1.0 | 94.04424 | 94.13553 | 93.42487 |
| 9 | 9 | 4.81 | 4.82 | 4.84 | 4.84 | 1 | 1 | 1 | 0.072543 | 0.237021 | 0.083521 | 0.237021 | 0.083521 | 0.237021 | 261 | 0.6 | 0.452426 | 1 | 94.03375 | 1.0 | 94.04424 | 94.13553 | 93.42487 |
| 10 | 10 | 4.81 | 4.82 | 4.84 | 4.84 | 1 | 1 | 1 | 0.072543 | 0.237021 | 0.083521 | 0.237021 | 0.083521 | 0.237021 | 261 | 0.6 | 0.452426 | 1 | 94.03375 | 1.0 | 94.04424 | 94.13553 | 93.42487 |
| 11 | 11 | 4.81 | 4.82 | 4.84 | 4.84 | 1 | 1 | 1 | 0.072543 | 0.237021 | 0.083521 | 0.237021 | 0.083521 | 0.237021 | 261 | 0.6 | 0.452426 | 1 | 94.03375 | 1.0 | 94.04424 | 94.13553 | 93.42487 |
| 12 | 12 | 4.81 | 4.82 | 4.84 | 4.84 | 1 | 1 | 1 | 0.072543 | 0.237021 | 0.083521 | 0.237021 | 0.083521 | 0.237021 | 261 | 0.6 | 0.452426 | 1 | 94.03375 | 1.0 | 94.04424 | 94.13553 | 93.42487 |
| 13 | 13 | 4.81 | 4.82 | 4.84 | 4.84 | 1 | 1 | 1 | 0.072543 | 0.237021 | 0.083521 | 0.237021 | 0.083521 | 0.237021 | 261 | 0.6 | 0.452426 | 1 | 94.03375 | 1.0 | 94.04424 | 94.13553 | 93.42487 |

Figure C.5: Raw Data from S/D=2.17, Unit weight=12.56 kN/m³

| S/D-L1P | # of PVC Pipe | Pipe Pressure Group 130% (Gage 130%) | Pipe Pressure Group 150% (Gage 150%) | Pipe Pressure Group 180% (Gage 180%) | Pipe Pressure Group 200% (Gage 200%) | Normalized Pipe Pressure Group 130% (130%) | Normalized Pipe Pressure Group 150% (150%) | Normalized Pipe Pressure Group 180% (180%) | Normalized Pipe Pressure Group 200% (200%) | SMA | PVC 544-1/2" Sd4 | | PVC Pipe Primary and Secondary Hoisting | | PVC Pipe Primary and Secondary Hoisting | | Flow Loss (Q ₀ /(Time/Length) (l/min/meter)) | Normalised Flow Loss | Logan Value | Normalised Log Value | Logan Value - Gage 13 | Logan Value - Gage 15 | Logan Value - Gage 18 | Logan Value - Gage 20 |
|---------|---------------|--------------------------------------|--------------------------------------|--------------------------------------|--------------------------------------|--|--|--|--|-----------|-----------------------------|------------------------|---|------------------------|---|------------------------|---|----------------------|-------------|----------------------|-----------------------|-----------------------|-----------------------|-----------------------|
| | | | | | | | | | | | Normalised Area/Gravel Area | Area Replacement Ratio | Normalised Gravel Area | Area Replacement Ratio | Normalised Gravel Area | Area Replacement Ratio | | | | | | | | |
| 8-01-0 | 0 | 5.385 | 5.269 | 5.254 | 5.221 | 1 | 1 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0.4243262 | 1 | 54.473026 | 1 | 54.473026 | 54.473026 | 54.473026 | 54.473026 |
| 8-01-1 | 3 | 5.194 | 5.178 | 5.234 | 5.192 | 0.9591865 | 0.9591865 | 0.9591865 | 0.9591865 | 0.1111111 | 0.2707521 | 0.0833221 | 0.1949265 | 0.2079221 | 0.2079221 | 0.2079221 | 0.4017951 | 0.9591865 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 |
| 8-01-2 | 4 | 5.088 | 5.171 | 5.203 | 5.159 | 0.9201820 | 0.9201820 | 0.9201820 | 0.9201820 | 0.1484648 | 0.3762138 | 0.0817562 | 0.1954513 | 0.2096238 | 0.2096238 | 0.2096238 | 0.4077911 | 0.9201820 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 |
| 8-01-3 | 5 | 4.922 | 5.101 | 5.147 | 5.097 | 0.8791779 | 0.8791779 | 0.8791779 | 0.8791779 | 0.1870202 | 0.4624845 | 0.0727626 | 0.1862582 | 0.2045242 | 0.2045242 | 0.2045242 | 0.4127141 | 0.8791779 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 |
| 8-01-4 | 6 | 4.822 | 5.101 | 5.147 | 5.097 | 0.8291847 | 0.8291847 | 0.8291847 | 0.8291847 | 0.2270272 | 0.5424845 | 0.0627626 | 0.1749265 | 0.1962242 | 0.1962242 | 0.1962242 | 0.4177141 | 0.8291847 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 |
| 8-01-5 | 7 | 4.691 | 4.774 | 4.713 | 4.706 | 0.8051205 | 0.8051205 | 0.8051205 | 0.8051205 | 0.2670272 | 0.6224845 | 0.0527626 | 0.1629265 | 0.1842242 | 0.1842242 | 0.1842242 | 0.4227141 | 0.8051205 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 |
| 8-01-6 | 8 | 4.527 | 4.61 | 4.621 | 4.596 | 0.7791948 | 0.7791948 | 0.7791948 | 0.7791948 | 0.3070272 | 0.7024845 | 0.0427626 | 0.1509265 | 0.1722242 | 0.1722242 | 0.1722242 | 0.4277141 | 0.7791948 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 |
| 8-01-7 | 9 | 4.321 | 4.408 | 4.454 | 4.408 | 0.7531901 | 0.7531901 | 0.7531901 | 0.7531901 | 0.3470272 | 0.7824845 | 0.0327626 | 0.1389265 | 0.1602242 | 0.1602242 | 0.1602242 | 0.4327141 | 0.7531901 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 |
| 8-01-8 | 10 | 4.100 | 4.187 | 4.233 | 4.187 | 0.7271854 | 0.7271854 | 0.7271854 | 0.7271854 | 0.3870272 | 0.8624845 | 0.0227626 | 0.1269265 | 0.1482242 | 0.1482242 | 0.1482242 | 0.4377141 | 0.7271854 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 |
| 8-01-9 | 11 | 4.008 | 4.095 | 4.141 | 4.100 | 0.7011807 | 0.7011807 | 0.7011807 | 0.7011807 | 0.4270272 | 0.9424845 | 0.0127626 | 0.1149265 | 0.1362242 | 0.1362242 | 0.1362242 | 0.4427141 | 0.7011807 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 |
| 8-01-10 | 12 | 4.013 | 4.100 | 4.146 | 4.100 | 0.6751760 | 0.6751760 | 0.6751760 | 0.6751760 | 0.4670272 | 1.0224845 | 0.0027626 | 0.1029265 | 0.1242242 | 0.1242242 | 0.1242242 | 0.4477141 | 0.6751760 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 |
| 8-01-11 | 13 | 4.04 | 4.231 | 4.277 | 4.231 | 0.6491713 | 0.6491713 | 0.6491713 | 0.6491713 | 0.5070272 | 1.1024845 | 0.0027626 | 0.0909265 | 0.1122242 | 0.1122242 | 0.1122242 | 0.4527141 | 0.6491713 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 | 54.473026 |

Figure C.10: Raw Data from S/D=2.17, Unit weight=13.03 kN/m³

| S/D-118 | | PVC Pipe Primary and Secondary Flowing | | | | PVC Pipe Primary and Secondary Stagnant | | | | PVC Pipe Primary and Secondary Stagnant | | | | PVC Pipe Primary and Secondary Stagnant | | | | | |
|----------------|----------------------------|--|----------------------------|----------------------------|---------------------------------------|---|---------------------------------------|---------------------------------------|---------------------------------------|---|---------------------------------------|---------------------------------------|---------------------------------------|---|---------------------------------------|---------------------------------------|---------------------------------------|---------------------------------------|---------------------------------------|
| # of PVC Pipes | Flow Pressure (Comp. 100%) | Flow Pressure (Comp. 100%) | Flow Pressure (Comp. 100%) | Flow Pressure (Comp. 100%) | Normalized Flow Pressure Gauge (100%) | Normalized Area Pressure Gauge (100%) | Normalized Area Pressure Gauge (100%) | Normalized Area Pressure Gauge (100%) | Normalized Area Pressure Gauge (100%) | Normalized Area Pressure Gauge (100%) | Normalized Area Pressure Gauge (100%) | Normalized Area Pressure Gauge (100%) | Normalized Area Pressure Gauge (100%) | Normalized Area Pressure Gauge (100%) | Normalized Area Pressure Gauge (100%) | Normalized Area Pressure Gauge (100%) | Normalized Area Pressure Gauge (100%) | Normalized Area Pressure Gauge (100%) | Normalized Area Pressure Gauge (100%) |
| | 5.34 | 5.76 | 5.36 | 5.36 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| N=1 | 0 | 3.73 | 3.87 | 3.87 | 0.701555 | 0.724522 | 0.727123 | 0.730209 | 0.733793 | 0.737378 | 0.740962 | 0.744546 | 0.748130 | 0.751714 | 0.755298 | 0.758882 | 0.762466 | 0.766050 | 0.769634 |
| N=2 | 3 | 5.07 | 5.63 | 5.26 | 0.867338 | 0.926259 | 0.920783 | 0.925307 | 0.930831 | 0.936355 | 0.941879 | 0.947403 | 0.952927 | 0.958451 | 0.963975 | 0.969499 | 0.975023 | 0.980547 | 0.986071 |
| N=3 | 4 | 4.98 | 5.08 | 4.94 | 0.947897 | 0.926467 | 0.926467 | 0.926467 | 0.926467 | 0.926467 | 0.926467 | 0.926467 | 0.926467 | 0.926467 | 0.926467 | 0.926467 | 0.926467 | 0.926467 | 0.926467 |
| N=4 | 5 | 4.80 | 4.92 | 4.94 | 0.921258 | 0.930352 | 0.931709 | 0.933066 | 0.934423 | 0.935780 | 0.937137 | 0.938494 | 0.939851 | 0.941208 | 0.942565 | 0.943922 | 0.945279 | 0.946636 | 0.947993 |
| N=5 | 6 | 4.62 | 4.72 | 4.52 | 0.887380 | 0.864802 | 0.864802 | 0.864802 | 0.864802 | 0.864802 | 0.864802 | 0.864802 | 0.864802 | 0.864802 | 0.864802 | 0.864802 | 0.864802 | 0.864802 | 0.864802 |
| N=6 | 7 | 3.96 | 4.02 | 4.32 | 0.833893 | 0.848326 | 0.848326 | 0.848326 | 0.848326 | 0.848326 | 0.848326 | 0.848326 | 0.848326 | 0.848326 | 0.848326 | 0.848326 | 0.848326 | 0.848326 | 0.848326 |
| N=7 | 8 | 4.12 | 4.22 | 4.52 | 0.888881 | 0.898881 | 0.898881 | 0.898881 | 0.898881 | 0.898881 | 0.898881 | 0.898881 | 0.898881 | 0.898881 | 0.898881 | 0.898881 | 0.898881 | 0.898881 | 0.898881 |
| N=8 | 9 | 3.89 | 4.07 | 4.15 | 0.842024 | 0.793285 | 0.793285 | 0.793285 | 0.793285 | 0.793285 | 0.793285 | 0.793285 | 0.793285 | 0.793285 | 0.793285 | 0.793285 | 0.793285 | 0.793285 | 0.793285 |
| N=9 | 10 | 3.73 | 3.87 | 3.87 | 0.701555 | 0.724522 | 0.727123 | 0.730209 | 0.733793 | 0.737378 | 0.740962 | 0.744546 | 0.748130 | 0.751714 | 0.755298 | 0.758882 | 0.762466 | 0.766050 | 0.769634 |
| N=10 | 11 | 3.47 | 3.51 | 3.98 | 0.662012 | 0.641629 | 0.641629 | 0.641629 | 0.641629 | 0.641629 | 0.641629 | 0.641629 | 0.641629 | 0.641629 | 0.641629 | 0.641629 | 0.641629 | 0.641629 | 0.641629 |
| N=11 | 12 | 3.24 | 3.33 | 3.43 | 0.628022 | 0.641259 | 0.641259 | 0.641259 | 0.641259 | 0.641259 | 0.641259 | 0.641259 | 0.641259 | 0.641259 | 0.641259 | 0.641259 | 0.641259 | 0.641259 | 0.641259 |
| N=12 | 13 | 3.10 | 3.16 | 3.29 | 0.593032 | 0.625249 | 0.625249 | 0.625249 | 0.625249 | 0.625249 | 0.625249 | 0.625249 | 0.625249 | 0.625249 | 0.625249 | 0.625249 | 0.625249 | 0.625249 | 0.625249 |
| N=13 | 14 | 2.90 | 3.01 | 3.04 | 0.558042 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 |
| N=14 | 15 | 2.94 | 2.97 | 2.96 | 0.548861 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 | 0.576249 |
| N=15 | 16 | 2.26 | 2.08 | 2.05 | 0.407244 | 0.388285 | 0.388285 | 0.388285 | 0.388285 | 0.388285 | 0.388285 | 0.388285 | 0.388285 | 0.388285 | 0.388285 | 0.388285 | 0.388285 | 0.388285 | 0.388285 |
| N=16 | 17 | 1.86 | 1.99 | 2.06 | 0.373253 | 0.373253 | 0.373253 | 0.373253 | 0.373253 | 0.373253 | 0.373253 | 0.373253 | 0.373253 | 0.373253 | 0.373253 | 0.373253 | 0.373253 | 0.373253 | 0.373253 |
| N=17 | 18 | 1.86 | 1.70 | 1.72 | 0.330264 | 0.313253 | 0.313253 | 0.313253 | 0.313253 | 0.313253 | 0.313253 | 0.313253 | 0.313253 | 0.313253 | 0.313253 | 0.313253 | 0.313253 | 0.313253 | 0.313253 |
| N=18 | 20 | 1.60 | 1.70 | 1.64 | 0.315264 | 0.321253 | 0.321253 | 0.321253 | 0.321253 | 0.321253 | 0.321253 | 0.321253 | 0.321253 | 0.321253 | 0.321253 | 0.321253 | 0.321253 | 0.321253 | 0.321253 |
| N=19 | 21 | 1.57 | 1.47 | 1.59 | 0.281881 | 0.278253 | 0.278253 | 0.278253 | 0.278253 | 0.278253 | 0.278253 | 0.278253 | 0.278253 | 0.278253 | 0.278253 | 0.278253 | 0.278253 | 0.278253 | 0.278253 |
| N=20 | 22 | 1.47 | 1.42 | 1.40 | 0.270249 | 0.270249 | 0.270249 | 0.270249 | 0.270249 | 0.270249 | 0.270249 | 0.270249 | 0.270249 | 0.270249 | 0.270249 | 0.270249 | 0.270249 | 0.270249 | 0.270249 |
| N=21 | 23 | 1.48 | 1.30 | 1.42 | 0.262271 | 0.262271 | 0.262271 | 0.262271 | 0.262271 | 0.262271 | 0.262271 | 0.262271 | 0.262271 | 0.262271 | 0.262271 | 0.262271 | 0.262271 | 0.262271 | 0.262271 |

Figure C.12: Raw Data from S/D=1.18, Unit weight=13.35 kN/m³

| S04-GR | | S05 | | PVC 34.4 by 5.6 | | PVC 40.1 by 5.6 | | PVC 40.1 by 5.6 | | PVC 40.1 by 5.6 | | PVC 40.1 by 5.6 | | PVC 40.1 by 5.6 | | PVC 40.1 by 5.6 | | PVC 40.1 by 5.6 | |
|----------------|--------------------------------------|--------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|------------------------------------|
| # of PVC Pipes | Deep Presum- Press-Gauge (kPa) | Deep Presum- Press-Gauge (kPa) | Normalized Press-Gauge (kPa) | Normalized Press-Gauge (kPa) | Normalized Press-Gauge (kPa) | Normalized Press-Gauge (kPa) | Normalized Press-Gauge (kPa) | Normalized Press-Gauge (kPa) | Normalized Press-Gauge (kPa) | Normalized Press-Gauge (kPa) | Normalized Press-Gauge (kPa) | Normalized Press-Gauge (kPa) | Normalized Press-Gauge (kPa) | Normalized Press-Gauge (kPa) | Normalized Press-Gauge (kPa) | Normalized Press-Gauge (kPa) | Normalized Press-Gauge (kPa) | Normalized Press-Gauge (kPa) | Normalized Press-Gauge (kPa) |
| 0 | 2.03 | 5.19 | 5.28 | 3.12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| 1 | 2.11 | 5.19 | 5.28 | 3.12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| 2 | 2.11 | 5.19 | 5.28 | 3.12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| 3 | 2.11 | 5.19 | 5.28 | 3.12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| 4 | 2.11 | 5.19 | 5.28 | 3.12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| 5 | 2.11 | 5.19 | 5.28 | 3.12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| 6 | 2.11 | 5.19 | 5.28 | 3.12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| 7 | 2.11 | 5.19 | 5.28 | 3.12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| 8 | 2.11 | 5.19 | 5.28 | 3.12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| 9 | 2.11 | 5.19 | 5.28 | 3.12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| 10 | 2.11 | 5.19 | 5.28 | 3.12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| 11 | 2.11 | 5.19 | 5.28 | 3.12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| 12 | 2.11 | 5.19 | 5.28 | 3.12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| 13 | 2.11 | 5.19 | 5.28 | 3.12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| 14 | 2.11 | 5.19 | 5.28 | 3.12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| 15 | 2.11 | 5.19 | 5.28 | 3.12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| 16 | 2.11 | 5.19 | 5.28 | 3.12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |
| 17 | 2.11 | 5.19 | 5.28 | 3.12 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 | 5.28 |

Figure C.14: Raw Data from S/D=1.62, Unit weight=13.35 kN/m³

| S/D=1.7 | Avg PVC Pipe | Pipe Pressure (Avg PVC) (kgf/cm ²) (kgf/cm ²) | Pipe Pressure (Avg PVC) (kgf/cm ²) (kgf/cm ²) | Avg Pipe Pressure (kgf/cm ²) (kgf/cm ²) | Normalized Pipe Pressure-Gauge (kgf/cm ²) (kgf/cm ²) | Normalized Pipe Pressure-Gauge (kgf/cm ²) (kgf/cm ²) | Normalized Pipe Pressure-Gauge (kgf/cm ²) (kgf/cm ²) | Normalized Average Pressure-Gauge (kgf/cm ²) (kgf/cm ²) | Normalized Average Pressure-Gauge (kgf/cm ²) (kgf/cm ²) | S4s | PVC Pipe-Primary and Secondary Footing | | PVC Pipe-Primary and Secondary Spacing of 0.20m of Pipe | | Flow Loss (Qout/Time)(Length) (L/min/feet) | Normalized Flow Loss | Average Lagor Value | Normalized Average Lagor Value | Lagor Value-Group 1 | Lagor Value-Group 2 | Lagor Value-Group 3 | Lagor Value-Group 4 |
|---------|--------------|---|---|---|--|--|--|---|---|-----------|--|--|--|--|--|----------------------|---------------------|--------------------------------|---------------------|---------------------|---------------------|---------------------|
| | | | | | | | | | | | Normalized Ground Area Multiplied by the Inverse of Area/Total | Area Replacement Ratio-Multiplied by the Inverse of Maximum Area/Total | Normalized Ground Area Multiplied by the Inverse of Area/Total | Area Replacement Ratio-Multiplied by the Inverse of Maximum Area/Total | | | | | | | | |
| Run 1 | 0 | 5.88 | 5.24 | 5.21 | 5.24 | 1 | 1 | 1 | 1 | 0 | 0 | 0 | 0 | 0.4920667 | 1 | 1.884209 | 1 | 96.287546 | 85.942071 | 81.888243 | 81.888243 | |
| Run 2 | 3 | 5.16 | 5.173 | 5.138 | 5.177 | 0.95939496 | 0.9023655 | 0.9284248 | 0.9284248 | 0.2370231 | 0.2370231 | 0.2370231 | 0.2370231 | 0.4827261 | 0.9023655 | 44.265832 | 1.0272757 | 94.675490 | 94.392949 | 94.392949 | 94.392949 | |
| Run 3 | 4 | 5.42 | 5.184 | 5.185 | 5.184 | 0.99202606 | 0.9840626 | 0.9878281 | 0.9878281 | 0.1933333 | 0.1933333 | 0.1933333 | 0.1933333 | 0.4837026 | 0.9840626 | 54.865246 | 1.0272757 | 94.583876 | 94.187608 | 94.187608 | 94.187608 | |
| Run 4 | 5 | 5.27 | 5.166 | 5.190 | 5.194 | 0.97894593 | 0.9704616 | 0.9746517 | 0.9746517 | 0.1654414 | 0.1654414 | 0.1654414 | 0.1654414 | 0.4920667 | 0.9704616 | 51.758945 | 0.9746517 | 94.359265 | 94.242007 | 94.242007 | 94.242007 | |
| Run 5 | 6 | 4.94 | 4.981 | 4.972 | 4.996 | 0.95939496 | 0.9515102 | 0.9654631 | 0.9654631 | 0.1452925 | 0.1452925 | 0.1452925 | 0.1452925 | 0.4920667 | 0.9515102 | 51.758945 | 0.9654631 | 94.187608 | 94.187608 | 94.187608 | 94.187608 | |
| Run 6 | 7 | 4.93 | 4.92 | 4.93 | 4.935 | 0.9899345 | 0.9877748 | 0.9938518 | 0.9938518 | 0.1382533 | 0.1382533 | 0.1382533 | 0.1382533 | 0.4920667 | 0.9877748 | 51.758945 | 0.9938518 | 94.187608 | 94.187608 | 94.187608 | 94.187608 | |
| Run 7 | 8 | 4.97 | 4.97 | 4.97 | 4.97 | 0.9899345 | 0.9877748 | 0.9938518 | 0.9938518 | 0.1382533 | 0.1382533 | 0.1382533 | 0.1382533 | 0.4920667 | 0.9877748 | 51.758945 | 0.9938518 | 94.187608 | 94.187608 | 94.187608 | 94.187608 | |
| Run 8 | 9 | 4.97 | 4.99 | 4.94 | 4.96 | 0.9899345 | 0.9877748 | 0.9938518 | 0.9938518 | 0.1382533 | 0.1382533 | 0.1382533 | 0.1382533 | 0.4920667 | 0.9877748 | 51.758945 | 0.9938518 | 94.187608 | 94.187608 | 94.187608 | 94.187608 | |
| Run 9 | 10 | 4.95 | 4.221 | 4.236 | 4.227 | 0.9899345 | 0.9877748 | 0.9938518 | 0.9938518 | 0.1382533 | 0.1382533 | 0.1382533 | 0.1382533 | 0.4920667 | 0.9877748 | 51.758945 | 0.9938518 | 94.187608 | 94.187608 | 94.187608 | 94.187608 | |
| Run 10 | 11 | 3.959 | 3.876 | 3.897 | 3.897 | 0.75918405 | 0.7435159 | 0.7448239 | 0.7448239 | 0.1452925 | 0.1452925 | 0.1452925 | 0.1452925 | 0.4920667 | 0.7435159 | 84.132597 | 0.987608 | 87.688407 | 86.652538 | 86.33797 | 86.33797 | |
| Run 11 | 12 | 3.95 | 3.538 | 3.539 | 3.537 | 0.69262692 | 0.6927293 | 0.6946639 | 0.6946639 | 0.1382533 | 0.1382533 | 0.1382533 | 0.1382533 | 0.4920667 | 0.6946639 | 84.132597 | 0.987608 | 87.688407 | 86.652538 | 86.33797 | 86.33797 | |
| Run 12 | 13 | 3.31 | 3.394 | 3.332 | 3.339 | 0.63331978 | 0.63331978 | 0.63331978 | 0.63331978 | 0.1382533 | 0.1382533 | 0.1382533 | 0.1382533 | 0.4920667 | 0.63331978 | 84.132597 | 0.987608 | 87.688407 | 86.652538 | 86.33797 | 86.33797 | |

Figure C.15: Raw Data from S/D=2.17, Unit weight=13.35 kN/m³

| S/D=1.8 | Raw Pressure - Post Pressure | | Normalized Post Pressure (kPa) | | Normalized Post Pressure (kPa) | | Normalized Average Post Pressure (kPa) | | Normalized Average Post Pressure (kPa) | | Normalized Average Post Pressure (kPa) | | Normalized Average Post Pressure (kPa) | | Normalized Average Post Pressure (kPa) | | Normalized Average Post Pressure (kPa) | |
|---------|------------------------------|---------------------|--------------------------------|---------------------|--------------------------------|---------------------|--|---------------------|--|---------------------|--|---------------------|--|---------------------|--|---------------------|--|---------------------|
| | Raw Pressure (kPa) | Post Pressure (kPa) | Raw Pressure (kPa) | Post Pressure (kPa) | Raw Pressure (kPa) | Post Pressure (kPa) | Raw Pressure (kPa) | Post Pressure (kPa) | Raw Pressure (kPa) | Post Pressure (kPa) | Raw Pressure (kPa) | Post Pressure (kPa) | Raw Pressure (kPa) | Post Pressure (kPa) | Raw Pressure (kPa) | Post Pressure (kPa) | Raw Pressure (kPa) | Post Pressure (kPa) |
| Raw 3 | 5.15 | 5.19 | 5.19 | 5.20 | 5.20 | 5.20 | 5.20 | 5.20 | 5.20 | 5.20 | 5.20 | 5.20 | 5.20 | 5.20 | 5.20 | 5.20 | 5.20 | 5.20 |
| Raw 4 | 4.97 | 5.01 | 4.99 | 4.96 | 4.96 | 4.96 | 4.96 | 4.96 | 4.96 | 4.96 | 4.96 | 4.96 | 4.96 | 4.96 | 4.96 | 4.96 | 4.96 | 4.96 |
| Raw 5 | 4.65 | 4.63 | 4.63 | 4.63 | 4.63 | 4.63 | 4.63 | 4.63 | 4.63 | 4.63 | 4.63 | 4.63 | 4.63 | 4.63 | 4.63 | 4.63 | 4.63 | 4.63 |
| Raw 6 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 |
| Raw 7 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 | 4.91 |
| Raw 8 | 4.39 | 4.52 | 4.52 | 4.52 | 4.52 | 4.52 | 4.52 | 4.52 | 4.52 | 4.52 | 4.52 | 4.52 | 4.52 | 4.52 | 4.52 | 4.52 | 4.52 | 4.52 |
| Raw 9 | 3.99 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 |
| Raw 10 | 3.99 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 |
| Raw 11 | 3.99 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 | 3.79 |
| Raw 12 | 3.11 | 3.19 | 3.19 | 3.19 | 3.19 | 3.19 | 3.19 | 3.19 | 3.19 | 3.19 | 3.19 | 3.19 | 3.19 | 3.19 | 3.19 | 3.19 | 3.19 | 3.19 |
| Raw 13 | 2.95 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 |
| Raw 14 | 2.95 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 | 3.00 |
| Raw 15 | 2.55 | 2.75 | 2.75 | 2.75 | 2.75 | 2.75 | 2.75 | 2.75 | 2.75 | 2.75 | 2.75 | 2.75 | 2.75 | 2.75 | 2.75 | 2.75 | 2.75 | 2.75 |
| Raw 16 | 2.87 | 2.62 | 2.62 | 2.62 | 2.62 | 2.62 | 2.62 | 2.62 | 2.62 | 2.62 | 2.62 | 2.62 | 2.62 | 2.62 | 2.62 | 2.62 | 2.62 | 2.62 |
| Raw 17 | 2.01 | 2.19 | 2.19 | 2.19 | 2.19 | 2.19 | 2.19 | 2.19 | 2.19 | 2.19 | 2.19 | 2.19 | 2.19 | 2.19 | 2.19 | 2.19 | 2.19 | 2.19 |
| Raw 18 | 1.65 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 |
| Raw 19 | 1.65 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 | 1.69 |
| Raw 20 | 1.65 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 |
| Raw 21 | 1.65 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 |
| Raw 22 | 1.65 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 |
| Raw 23 | 1.65 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 |
| Raw 24 | 1.65 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 |
| Raw 25 | 1.65 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 | 1.50 |

Figure C.17: Raw Data from S/D=1.18, Unit weight=13.82 kN/m³

| # of PVC Pipe | PVC Pipe ID | PVC Pipe Length (m) | PVC Pipe Weight (kg) | PVC Pipe Volume (m ³) | PVC Pipe Density (kg/m ³) | 3.0m | | PVC Pipe Weight Sectional Pricing | | PVC Pipe Volume Sectional Pricing | | Row Loss (Qu/Trm Length) (L/Trm/Inch) | Length (m) | Row Loss (Qu/Trm Length) (L/Trm/Inch) | Normalized Row Loss | Average Upstream Value | Normalized Average Upstream Value | Ligens Value Gauge 12 | Ligens Value Gauge 13 | Ligens Value Gauge 14 | |
|---------------|-------------|---------------------|----------------------|-----------------------------------|---------------------------------------|-----------------------------------|------------------------|-----------------------------------|------------------------|-----------------------------------|------------------------|---------------------------------------|------------|---------------------------------------|---------------------|------------------------|-----------------------------------|-----------------------|-----------------------|-----------------------|----|
| | | | | | | Normalized Area (m ²) | Area (m ²) | Normalized Area (m ²) | Area (m ²) | Normalized Area (m ²) | Area (m ²) | | | | | | | | | | |
| 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | |
| 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 |
| 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 | 4 |
| 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 |
| 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 | 6 |
| 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 |
| 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 | 8 |
| 9 | 9 | 9 | 9 | 9 | 9 | 9 | 9 | 9 | 9 | 9 | 9 | 9 | 9 | 9 | 9 | 9 | 9 | 9 | 9 | 9 | 9 |
| 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 |
| 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 |
| 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 |
| 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 |
| 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 |
| 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 |
| 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 |
| 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 |
| 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 |
| 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 |
| 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 |

Figure C.18: Raw Data from S/D=1.45, Unit weight=13.82 kN/m³

| S/D# | # of PVC Pipe | Non Pressure | | Pipe Pressure | | Avg. Pipe Pressure (kPa) | Pipe Pressure (kPa) | Normalized Pipe Pressure-Gauge (130k) | Normalized Pipe Pressure-Gauge (120k) | Normalized Pipe Pressure-Gauge (140k) | Normalized Average Pipe Pressure (kPa) | S/D | PVC 661 PVC 6 | | PVC 661 PVC 67 | | PVC 661 PVC 68 | | Pipe Us (Bar/Time/Length) (mm/min) | Normalized Flow us | Average Input/Value | Normalized Average Input/Value | Ligpen Value Gauge 1 | Ligpen Value Gauge 2 | Ligpen Value Gauge 3 | Ligpen Value Gauge 4 | | |
|------|---------------|---------------|---------------|---------------|---------------|--------------------------|---------------------|---------------------------------------|---------------------------------------|---------------------------------------|--|-----|---------------|---------------|----------------|---------------|----------------|--|------------------------------------|--------------------|---------------------|--------------------------------|----------------------|----------------------|----------------------|----------------------|--|--|
| | | Group 1 (kPa) | Group 2 (kPa) | Group 1 (kPa) | Group 2 (kPa) | | | | | | | | Group 1 (kPa) | Group 2 (kPa) | Group 1 (kPa) | Group 2 (kPa) | | | | | | | | | | | | |
| 0 | 5.24 | 5.24 | 5.24 | 5.24 | 5.24 | 5.24 | | | | | | | | | | | | | | | | | | | | | | |
| 1 | 5.12 | 5.12 | 5.12 | 5.12 | 5.12 | 5.12 | | | | | | | | | | | | | | | | | | | | | | |
| 2 | 4.96 | 4.96 | 4.96 | 4.96 | 4.96 | 4.96 | | | | | | | | | | | | | | | | | | | | | | |
| 3 | 4.79 | 4.79 | 4.79 | 4.79 | 4.79 | 4.79 | | | | | | | | | | | | | | | | | | | | | | |
| 4 | 4.52 | 4.52 | 4.52 | 4.52 | 4.52 | 4.52 | | | | | | | | | | | | | | | | | | | | | | |
| 5 | 4.26 | 4.26 | 4.26 | 4.26 | 4.26 | 4.26 | | | | | | | | | | | | | | | | | | | | | | |
| 6 | 4.00 | 4.00 | 4.00 | 4.00 | 4.00 | 4.00 | | | | | | | | | | | | | | | | | | | | | | |
| 7 | 3.74 | 3.74 | 3.74 | 3.74 | 3.74 | 3.74 | | | | | | | | | | | | | | | | | | | | | | |
| 8 | 3.48 | 3.48 | 3.48 | 3.48 | 3.48 | 3.48 | | | | | | | | | | | | | | | | | | | | | | |
| 9 | 3.22 | 3.22 | 3.22 | 3.22 | 3.22 | 3.22 | | | | | | | | | | | | | | | | | | | | | | |
| 10 | 2.96 | 2.96 | 2.96 | 2.96 | 2.96 | 2.96 | | | | | | | | | | | | | | | | | | | | | | |
| 11 | 2.70 | 2.70 | 2.70 | 2.70 | 2.70 | 2.70 | | | | | | | | | | | | | | | | | | | | | | |
| 12 | 2.44 | 2.44 | 2.44 | 2.44 | 2.44 | 2.44 | | | | | | | | | | | | | | | | | | | | | | |
| 13 | 2.18 | 2.18 | 2.18 | 2.18 | 2.18 | 2.18 | | | | | | | | | | | | | | | | | | | | | | |
| 14 | 1.92 | 1.92 | 1.92 | 1.92 | 1.92 | 1.92 | | | | | | | | | | | | | | | | | | | | | | |
| 15 | 1.66 | 1.66 | 1.66 | 1.66 | 1.66 | 1.66 | | | | | | | | | | | | | | | | | | | | | | |
| 16 | 1.40 | 1.40 | 1.40 | 1.40 | 1.40 | 1.40 | | | | | | | | | | | | | | | | | | | | | | |
| 17 | 1.14 | 1.14 | 1.14 | 1.14 | 1.14 | 1.14 | | | | | | | | | | | | | | | | | | | | | | |

Figure C.19: Raw Data from S/D=1.62, Unit weight=13.82 kN/m³

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VITA

Elliot Nicholas Magoto graduated from the University of Kentucky with a Bachelors of Science in Civil Engineering. He graduated on December 2012 with Magna Cum Laude honors. Elliot passed the Fundamentals of Engineering exam and was awarded the Engineer in Training certificate in the Spring of 2012.

Personal Publications:

Magoto, E.N. and Bryson, L.S.(2013), “Evaluation of the Effectiveness of a Grout Curtain using a Physical Model”, *Dam Safety 2013*, The Association of State Dam Safety Officials, Providence, RI, 8-12 September, 2013.

“Evaluation of the Effectiveness of a Grout Curtain using a Physical Model”, *Dam Safety 2013*, The Association of State Dam Safety Officials, *Poster Presentation*. Providence, RI, 8-12 September, 2013.